

July 2, 2025

Tim Pearson
Conservation Agent
Littleton Conservation Commission
37 Shattuck Street
P.O. Box 1305
Littleton, MA 01460

RE: Request for Amendment to Order of Conditions for DEP#204-0991,
The Reconstruction of Foster Street, MassDOT Project #609054
Littleton, MA.

Attached:

- Groundwater test pit results summary
- Alternatives analysis for replacement of leaching catch basins
- Plan set markup and detail of crushed stone diaphragm trench
- Equivalency calculations for leaching catch basins and crushed stone diaphragm trench

CC: Stephen Jahnle, DPW Director, Town of Littleton; Chris Deloge, Resident Engineer, MassDOT

Dear Commission Members:

On behalf of the Town of Littleton Department of Public Works director Stephen Jahnle, we are requesting an Amended Order of Conditions for the Reconstruction of Foster Street Project (MassDEP File No. 204-0991). The Order of Conditions (OOC) was issued on December 12, 2023. The request is for an allowance for the reduction in the proposed number of leaching catch basin structures from 5 to 1 due to the discovery of high seasonal groundwater throughout the project limits during construction. To replace the stormwater storage volume that the 4 leaching catch basins would have provided, a 100ft crushed stone diaphragm is now proposed at the roadside of Foster Street to offset the loss of the 4 leaching catch basins.

Following up from our last appearance before the commission on 6/10/2025. We have performed the following actions:

- 3 additional groundwater test pits were performed within the Foster Street corridor. The total of groundwater test pits performed on site now total 10 distributed throughout the 0.7-mile corridor.
- An alternatives analysis narrative has been created describing stormwater infiltration options to replace the 4 leaching catch basins.

- Prepared revised plans, details, and volumetric calculations showing a 100ft long crushed stone diaphragm trench along the side of Foster Street designed to provide an equivalent amount of stormwater storage for infiltration as compare to the 4 leaching catch basins that could not be installed.

Background

The project was permitted as a limited project and a redevelopment project for road widening less than one lane due to the addition of a paved shared-use path. As a redevelopment project, Stormwater standard #7 was responded to by the following in the NOI, “This project is a redevelopment project. Standards 2, 3, and 4 are met to the maximum extent practicable. Standards 1, 8, 9, and 10 are met fully”.

The project plans originally included 5 MassDOT standard underground leaching cement concrete catch basin structures, 4.0' x 6.5' in dimensional size, located along the back of curb on Foster Street. The 5 leaching catch basins were each connected to a proposed new traditional deep sump catch basin for added groundwater recharge. Three of the leaching catch basins were located inside of the 100' buffer area to BVW. Design guidelines require that seasonal high groundwater be at least a minimum 2ft below the bottom of the proposed leaching catch basin structures for stormwater infiltration.

The project did not include groundwater elevation testing during the design phase due to the invasive nature of the tests adjacent to or in some cases within a public roadway. Groundwater test pits were performed during construction to determine estimated seasonal high groundwater elevation by a licensed soil evaluator in Massachusetts. The results of the groundwater test pits have shown that seasonal high groundwater elevations are too high for the installation of the proposed leaching catch basins at each the original proposed 5 locations. See attached markup of the project plans showing the location of the groundwater test pits and a memo from Dillis & Roy, a firm specializing in soil evaluation, describing the estimated seasonal high groundwater results at each test pit location.

At nine of the ten test pit locations, the estimated seasonal high groundwater was 5.0ft or less in depth from the ground surface. Neither a standard MassDOT leaching catch basin nor the smallest size available leaching basin structure, a mini-leaching catch basin, are feasible for installation. The standard MassDOT leaching catch basins requires a minimum of 8.5 feet of seasonal high groundwater depth, while the mini-leaching catch basin would require a minimum of 5.9 feet in depth. See attached construction details of the available leaching basin structures.

One test pit location, Test Pit-33, had an estimated high seasonal groundwater of 9ft in depth. This is enough depth for the installation of a standard MassDOT catch basin. One new proposed leaching basin is proposed here along the southerly curbline Foster Street, approximately 200ft west of Balsam Lane. The location is outside of the 100ft buffer area to BVW. In summary, 5 leaching catch basins were proposed in the original permitting plans, however one leaching catch basin is now proposed due to the discovery of high groundwater throughout most of the project corridor.

Abutter Notification

An abutter's list was sourced from the Town of Littleton assessor's office including abutters that are 1,000ft from the linear project limits in accordance with M.G.L. Chapter 131: Section 40. Notification was sent the day of this request.

Sincerely,



Aaron Keegan, PE
Project Engineer
Fuss & O'Neill, Inc.
(413) 333-5461
aaron.keegan@fando.com

GROUNDWATER TEST PIT RESULTS SUMMARY DATA

Groundwater Elevation Test Method Memorandum

JN 8313

Date: September 26, 2024

Revised: June 20, 2025

To: Bryan Blackerby, Onyx Corporation

From: Francis McPartlan, PE (MA)

CC: Jack Maloney, Dillis & Roy Civil Design Group

RE: Ground Water Elevation Test Results
Foster Street
Littleton, Massachusetts

Jack Maloney, Massachusetts Soil Evaluator Number SE13704, determined the depth to estimated seasonal high groundwater (ESHGW) at four (4) test pits. The test pits were excavated by Onyx Corporation using vacuum excavation methodology on September 26, 2024.

Test Pit	Total Depth	ESHGW
TP-22	44 inches	36 inches
TP-24	44 inches	34 inches
TP-25	46 inches	38 inches
TP-27	70 inches	60 inches

Additional test pits were excavated by Onyx Corporation using vacuum excavation methodology on October 25, 2024.

Test Pit	Total Depth	ESHGW
TP-32	38 inches	36 inches
TP-33	108 inches	>108 inches

An additional test pit was excavated by Onyx Corporation using vacuum excavation methodology on November 13, 2024.

Test Pit	Total Depth	ESHGW
TP-6+80	48 inches	48 inches

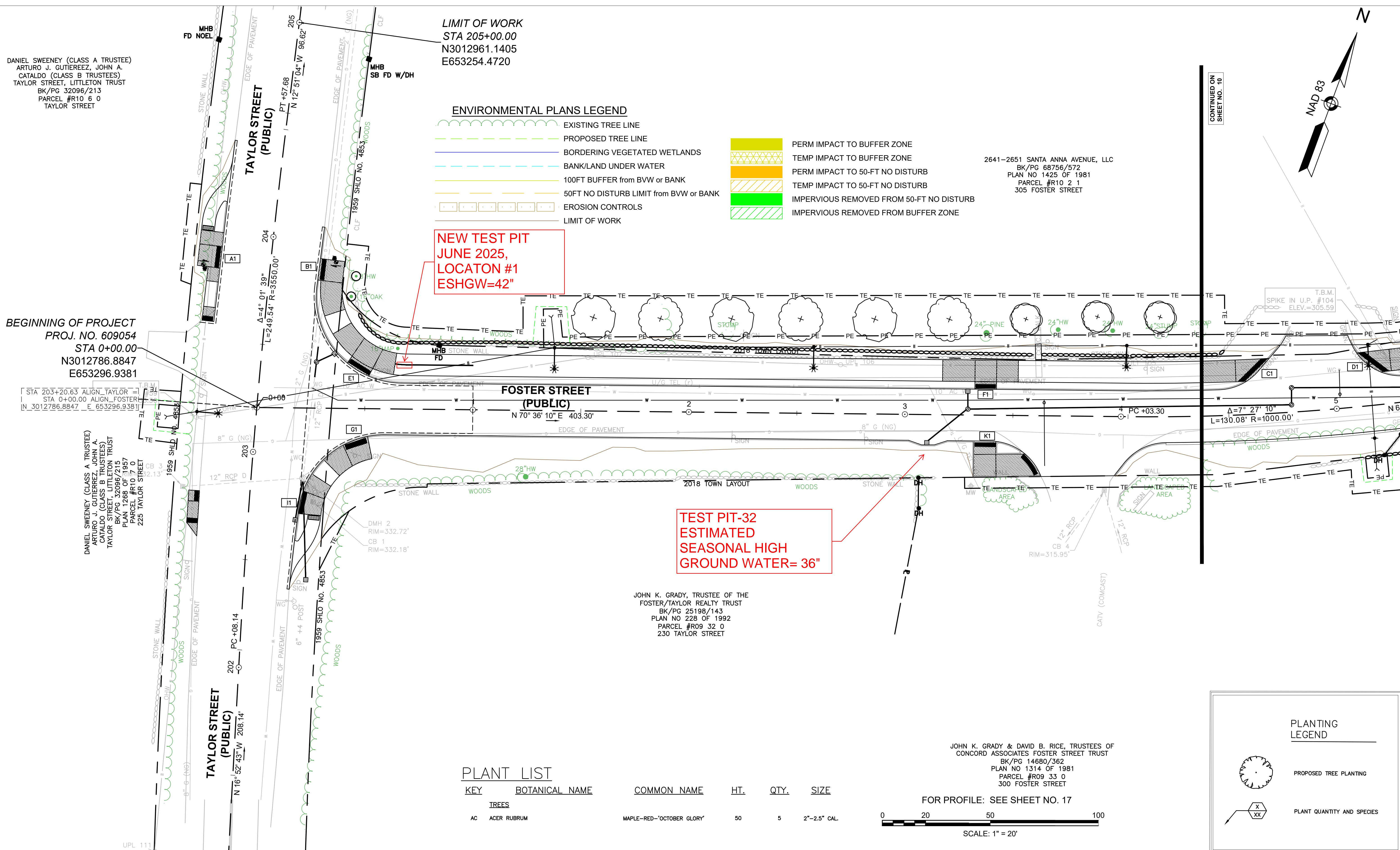
Three additional test pits were excavated by Onyx Corporation using vacuum excavation methodology on June 20, 2025. TP-1 is located at STA 0+70, 20' left at the intersection of Foster Street and Taylor Street. TP-2 is located at STA 34+28, 20' left on Foster Street. TP-3 is located at STA 35+80, 20' left.

Test Pit	Total Depth	ESHGW	Refusal
TP-1	55 inches	42 inches	N/A
TP-2	18 inches	N/A	18 inches
TP-3	36 inches	N/A	36 inches

MASSDOT 125644 - RECONSTRUCTION OF FOSTER STREET - LITTLETON, MA
TEST PIT EXPLORATION PLAN / TRACKING RECAP

Updated 9/17/2024

TEST PIT #	LOCATION	DESCRIPTION	DWG SHEET NO.	NOTES / COMMENTS	WIDTH	LENGTH	DEPTH	TOTAL CY	DATE PERFORMED
TP-01	STA 3+29.29, 11.0 R	PROP. TEST PIT - GAS , DRAINAGE	57	PR-CB-03: 2'-4" DEEP FROM TOP OF ASPHALT 8" PLASTIC GAS; 2'-4" FROM CENTER OF CB TO GAS	1.00	1.00	2.33	0.09	9/4/2024
TP-02	STA 6+81.35, 11.0 R	PROP. TEST PIT - GAS, PROP. WATER & PROP. DRAINAGE	58	PR-CB-04: 2'-3" DEEP FROM TOP OF ASPHALT TO GAS; APPEARS CB4 WILL FIT	1.00	1.00	2.25	0.08	9/4/2024
TP-03	STA 7+11.5, 4.25 RT	PROP. TEST PIT - GAS, PROP. WATER, AND CB	58	2'-3" DEEP FROM TOP OF ASPHALT TO GAS	1.00	1.00	2.25	0.08	9/4/2024
TP-04	STA 8+65.5, 12.25 RT	PROP. TEST PIT - GAS	58	2'-7" DEEP FROM TOP OF ASPHALT TO GAS	1.00	1.00	2.58	0.10	9/4/2024
TP-05	STA 10+29.83, 3.83 RT	PROP. TEST PIT - GAS, PROP. WATER , DRAIN	59	GAS: 8" PLASTIC GAS 10FT OFF FROM PROPOSED TEST PIT, 3FT DOWN FROM TOP OF ASPHALT	2.00	2.00	3.50	0.52	9/16/2024
TP-06	STA 13+92.42, 13.00 RT	PROP. TEST PIT - GAS	59	GAS: 2'-7" DEEP FROM TOP OF ASPHALT TO GAS	3.00	3.00	2.50	0.83	9/4/2024
TP-07	STA 16+81.33, 2.37 RT	PROP. TEST PIT - GAS & PROP. DRAIN	60	REMODEL EX-CB-18 - NEW RCP	1.80	1.50	3.00	0.30	9/4/2024
TP-08	STA 22+60.33, 8.85 RT	PROP. TEST PIT - GAS & PROP. DRAIN	61	PR-CB-09	1.50	1.50	2.30	0.19	9/4/2024
TP-09	STA 23+98.33, 5.08 RT	PROP. TEST PIT - GAS & PROP. DRAIN	61	PR-CB-11 & RCP: 2'-4" DEEP FROM TOP OF ASPHALT TO GAS	1.00	1.00	2.30	0.09	9/4/2024
TP-10	STA 25+87.04, 5.19 RT	PROP. TEST PIT - GAS	62	PR-CB-12 & RCP: 3" STEEL FOUND 2'-8" DEEP FROM TOP OF ASPHALT	1.00	1.00	2.67	0.10	9/4/2024
TP-11	STA 26+32.25, 4.62 RT	PROP. TEST PIT - GAS	62	3" STEEL FOUND 3FT DOWN FROM TOP OF ASPHALT	1.50	1.50	3.00	0.25	9/4/2024
TP-12	STA 31+69.06, 6.77 RT	PROP. TEST PIT - GAS & PROP. DRAIN	63	REMODEL EX-DMH-12 - NEW RCP: GAS 4'-3" FROM TOP OF ASPHALT, DRAIN 2'-10" DOWN; WATER 5' DOWN	3.00	3.00	5.00	1.67	9/5/2024
TP-13	STA 35+94.04, 6.85 RT	PROP. TEST PIT - GAS & PROP. DRAIN	64	PR-CB-17: GAS LINE 3'-2" DOWN FROM TOP OF ASPHALT 5'-6" OFF OF PROPOSED TESET PIT (OFF EDGE OF ROAD)	1.50	1.50	3.17	0.26	9/5/2024
TP-14	STA 38+00, 10.5 RT	PROP. TEST PIT - GAS & PROP. WATER	64	GAS 3'-5" DOWN FROM TO OF ASPHALT; WATER 5'-7" DOWN FROM TOP OF ASPHALT (10" DI PIPE)	1.50	1.50	5.58	0.47	9/5/2024
TP-15	STA 39+10.17, 8.25 RT	PROP. TEST PIT - GAS & PROP. WATER	64	PROPOSED HYDRANT: GAS 3' DOWN FROM TOP OF ASPHALT DIRECTLY UNDER HMA BERM; WATER NOT FOUND - NEED LWD TO LAYOUT	2.00	5.00	7.00	2.59	9/5/2024
TP-16	STA 39+25.5, 8.92 RT	PROP. TEST PIT - WATER, GAS	64	WATER MAIN 5FT DOWN FROM TOP OF ASPHALT; DIRECTLY UNDER EDGE OF ROAD	2.50	2.50	5.50	1.27	9/16/2024
TP-17	STA 35+96.5, 7.08 LT	PROP. TEST PIT - GAS, DRAINAGE	64	PR-GI-02: GAS FOUND 3' DOWN FROM TOP OF ASPHALT	1.50	1.50	3.00	0.25	9/5/2024
TP-18	STA 31+66.5, 12.58 LT	PROP. TEST PIT - GAS & PROP. DRAIN	63	PR-CB-14: GAS FOUND 3'-9" DOWN FROM TOP OF ASPHALT	1.50	1.50	4.00	0.33	9/5/2024
TP-19	STA 30+71.2, 15.83 LT	PROP. TEST PIT - GAS	63	PROPOSED GRANITE CURB: 2" STEEL LINE FOUND 2'-8" DOWN FROM TOP OF ASPHALT, 8" STEEL LINE FOUND 3'9" DOWN FROM TOP OF ASPHALT	1.50	1.50	4.00	0.33	9/5/2024
TP-20	STA 29+65.8, 15.67 LT	PROP. TEST PIT - GAS, CURB	62	PROPOSED GRANITE CURB: 2" STEEL LINE FOUND 2'-8" DOWN FROM TOP OF ASPHALT, 8" STEEL LINE FOUND 3'-5" DOWN FROM TOP OF ASPHALT	1.50	1.50	3.50	0.29	9/5/2024
TP-21	STA 26+36.1, 19.04 LT	PROP. TEST PIT - GAS	62	GAS: 12" STEEL PIPE 2'-10" DOWN FROM TOP OF ASPHALT	2.00	2.00	3.00	0.44	9/16/2024
TP-22	STA 25+88.81, 20.83 LT	TEST PIT - GROUNDWATER ELEVATION	62	LB-05: NO GROUNDWATER ENCOUNTERED; PREVIOUS TOP OF GROUNDWATER MEASUREMENT 3FT FROM TOP OF ASPHALT.	2.00	2.00	3.67	0.54	9/16/2024
TP-23	STA 25+23.75, 8.17 LT	PROP. TEST PIT	62	STREET ELEVATION 236.45. 3" ST LINE 2'-6" DEEP FROM TOP OF ASPHALT. 9'-3" DEAD ENDED FROM FACE OF CURB	3.00	3.00	3.00	1.00	9/4/2024
TP-24	STA 22+59.77, 21.25 LT	TEST PIT - GROUNDWATER ELEVATION	61	LB-04: NO GROUNDWATER ENCOUNTERED; PREVIOUS TOP OF GROUNDWATER MEASUREMENT 3FT FROM TOP OF ASPHALT.	2.00	2.00	3.67	0.54	9/16/2024
TP-25	STA 16+89.53, 21.67 LT	TEST PIT - GROUNDWATER ELEVATION	60	LB-03: NO GROUNDWATER ENCOUNTERED; PREVIOUS TOP OF GROUNDWATER MEASUREMENT 3'-2" FROM TOP OF ASPHALT.	2.00	2.00	3.83	0.57	9/16/2024
TP-26	STA 14+11.89, 12.25 LT	TEST PIT - WATER, DRAIN	59	PR-DMH-05: WATER PIPE 5FT DOWN TO TOP OF PIPE; DRAINAGE PIPE 2'-4" TO TOP OF PIPE	2.00	2.00	5.33	0.79	9/16/2024
TP-27	STA 14+04.57, 20.83 LT	TEST PIT - GROUNDWATER ELEVATION	59	LB-02: NO GROUNDWATER ENCOUNTERED; PREVIOUS TOP OF GROUNDWATER MEASUREMENT 5FT FROM TOP OF ASPHALT.	2.00	2.00	5.83	0.86	9/16/2024
TP-28	STA 6+81.56, 14.58 LT	TEST PIT - WATER & PROP. DRAIN	58	PR-CBDB-01 - LOCATED ON BACKSIDE OF BERM - 5.25FT DEEP; STREET ELEVATION 209.06	2.00	4.00	5.25	1.56	9/3/2024
TP-29	STA 6+79.86, 22.08 LT	F&C TEST PIT REQUIRED FOR GROUNDWATER ELEVATION	58	LB-01 - NO GROUNDWATER FOUND	1.00	1.00	6.00	0.22	9/3/2024
TP-30	STA 3+31.83, 6.83 LT	PROP. TEST PIT - WATER & PROP. DRAIN	57	PR-GI-01 - 4.5 FT DEEP 12" AC MAIN	2.00	4.00	4.50	1.33	9/3/2024
TP-31	STA 3+9.25, 15.5 LT	PROP. TEST PIT - TELECOM, CURB	57	PROPOSED CURBING - 4.5' DEEP TWO (2) DIRECT BURIAL LINES\	1.00	1.00	4.50	0.17	9/3/2024



ENVIRONMENTAL PLANS LEGEND

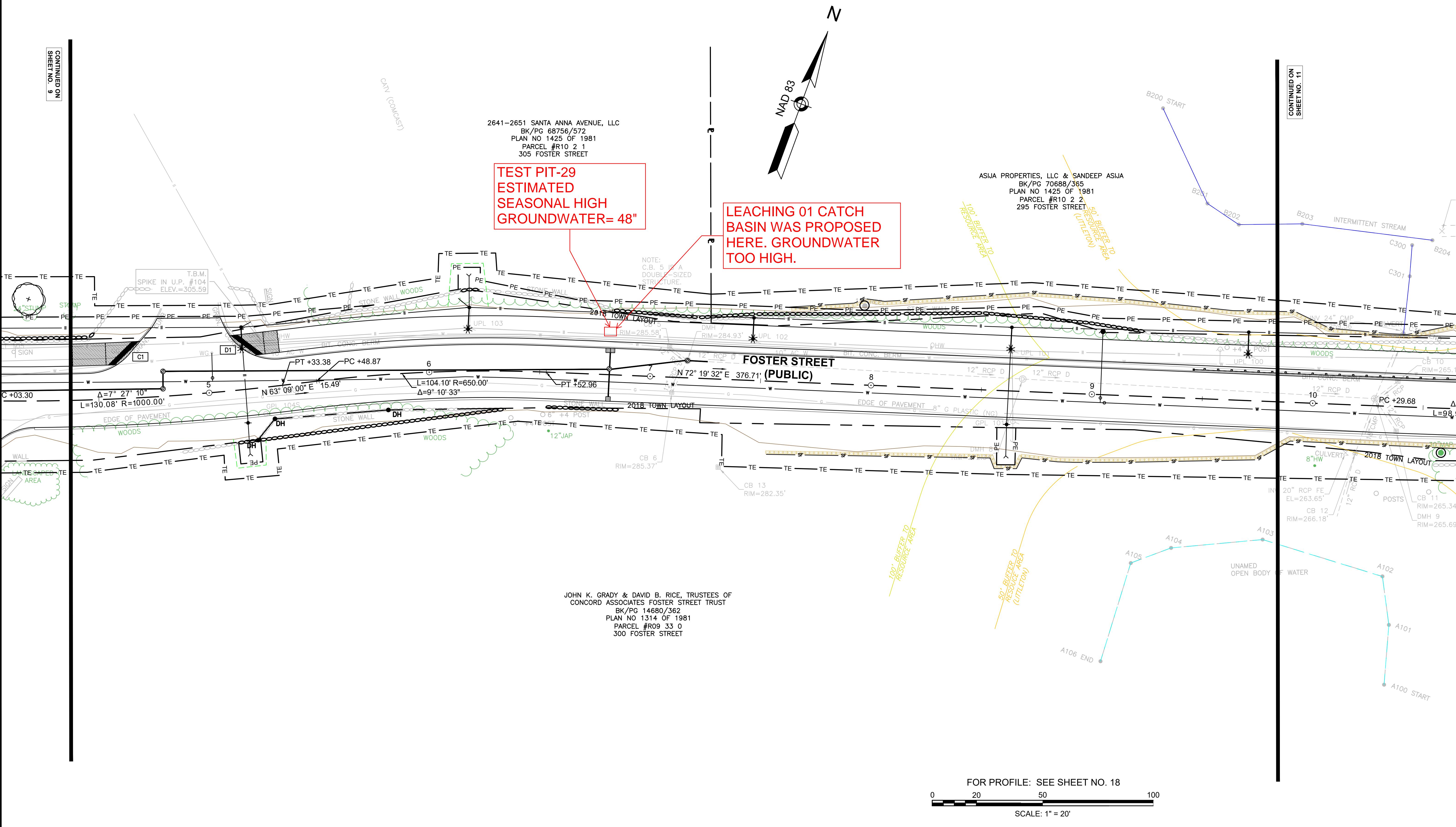
	EXISTING TREE LINE
	PROPOSED TREE LINE
	BORDERING VEGETATED WETLANDS
	BANK/LAND UNDER WATER
	100FT BUFFER from BVW or BANK
	50FT NO DISTURB LIMIT from BVW or BANK
	EROSION CONTROLS
	LIMIT OF WORK
	PERM IMPACT TO BUFFER ZONE
	TEMP IMPACT TO BUFFER ZONE
	PERM IMPACT TO 50-FT NO DISTURB
	TEMP IMPACT TO 50-FT NO DISTURB
	IMPERVIOUS REMOVED FROM 50-FT NO DISTURB
	IMPERVIOUS REMOVED FROM BUFFER ZONE

LITTLETON
RECONSTRUCTION OF FOSTER STREET

STATE	FED. AID PROJ. NO.	HEET NO.	TOTAL SHEETS
MA	STP/CMQ/TAP-003(037)X	E2	128
PROJECT FILE NO. 609054			

ENVIRONMENTAL PLANS

20170044A21.HPN01 NOI SET TEST PTS.DWG Plotted on 5-Jun-2025 4:16 PM



ENVIRONMENTAL PLANS LEGEND

	EXISTING TREE LINE
	PROPOSED TREE LINE
	BORDERING VEGETATED WETLANDS
	BANK/LAND UNDER WATER
	100FT BUFFER from BVW or BANK
	50FT NO DISTURB LIMIT from BVW or BANK
	EROSION CONTROLS
	LIMIT OF WORK
	PERM IMPACT TO BUFFER ZONE
	TEMP IMPACT TO BUFFER ZONE
	PERM IMPACT TO 50-FT NO DISTURB
	TEMP IMPACT TO 50-FT NO DISTURB
	IMPERVIOUS REMOVED FROM 50-FT NO DISTURB
	IMPERVIOUS REMOVED FROM BUFFER ZONE

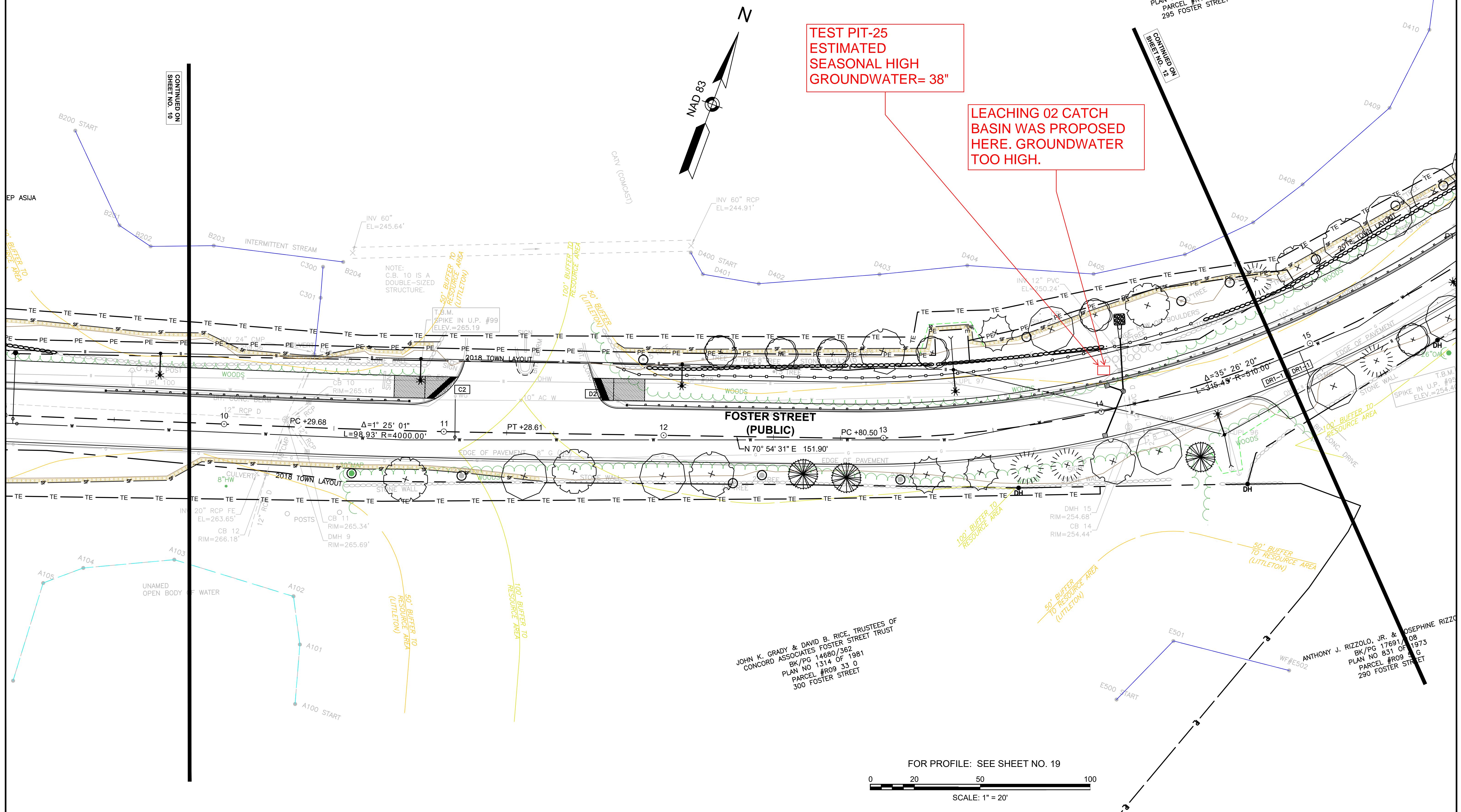
LITTLETON
RECONSTRUCTION OF FOSTER STREET

STATE	FED. AID PROJ. NO.	HEET NO.	TOTAL SHEETS
MA	STP/CMQ/TAP-0033(037)X	E3	128
PROJECT FILE NO. 609054			

ENVIRONMENTAL PLANS

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ENVIRONMENTAL PLANS LEGEND

EXISTING TREE LINE

PROPOSED TREE LINE

BORDERING VEGETATED WETLANDS

BANK/LAND UNDER WATER

100FT BUFFER from BVW or BANK

50FT NO DISTURB LIMIT from BVW or BANK

EROSION CONTROLS

LIMIT OF WORK

	PERM IMPACT TO BUFFER ZONE
	TEMP IMPACT TO BUFFER ZONE
	PERM IMPACT TO 50-FT NO DISTURB
	TEMP IMPACT TO 50-FT NO DISTURB
	IMPERVIOUS REMOVED FROM 50-FT NO DISTURB
	IMPERVIOUS REMOVED FROM BUFFER ZONE

LITTLETON

RECONSTRUCTION OF FOSTER STREET

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP/CMQ/TAP-0033(037)X	E4	128
PROJECT FILE NO.		609054	

ENVIRONMENTAL PLANS

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ASIJAH PROPERTIES, LLC & SANDEEP ASIJAH
BK/PG 70688/365
PLAN NO 1425 OF 1981
PARCEL #R10 2 2
295 FOSTER STREET

**TEST PIT-25
ESTIMATED
SEASONAL HIGH
GROUNDWATER= 38**

LEACHING 03 CATCH
BASIN WAS PROPOSE
HERE. GROUNDWATE
TOO HIGH.

**CONTINUED ON
SHEET NO. 13**

JORDAN S. MCCATHERIN
BK/PG 62917/52
PARCEL #R10 1 0
277 FOSTER STREET

ENVIRONMENTAL PLANS LEGEND

Legend for the site plan:

- EXISTING TREE LINE (Green wavy line)
- PROPOSED TREE LINE (Green dashed line)
- BORDERING VEGETATED WETLANDS (Blue solid line)
- BANK/LAND UNDER WATER (Cyan dashed line)
- 100FT BUFFER from BVW or BANK (Yellow solid line)
- 50FT NO DISTURB LIMIT from BVW or BAN (Orange solid line)
- EROSION CONTROLS (Yellow boxes)
- LIMIT OF WORK (Red line)

	PERM IMPACT TO BUFFER ZONE
	TEMP IMPACT TO BUFFER ZONE
	PERM IMPACT TO 50-FT NO DISTURB
	TEMP IMPACT TO 50-FT NO DISTURB
	IMPERVIOUS REMOVED FROM 50-FT NO DISTURB
	IMPERVIOUS REMOVED FROM BUFFER ZONE

LITTLETON

RECONSTRUCTION OF FOSTER STREET

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP/CMQ/TAP-0033(037)X	E5	128
PROJECT FILE NO.		609054	

ENVIRONMENTAL PLANS

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TEST PIT-24
ESTIMATED SEASONAL HIGH GROUNDWATER= 34"

LEACHING 04 CATCH BASIN WAS PROPOSED HERE. GROUNDWATER TOO HIGH.

FOR PROFILE: SEE SHEET NO. 21

SCALE: 1" = 20'

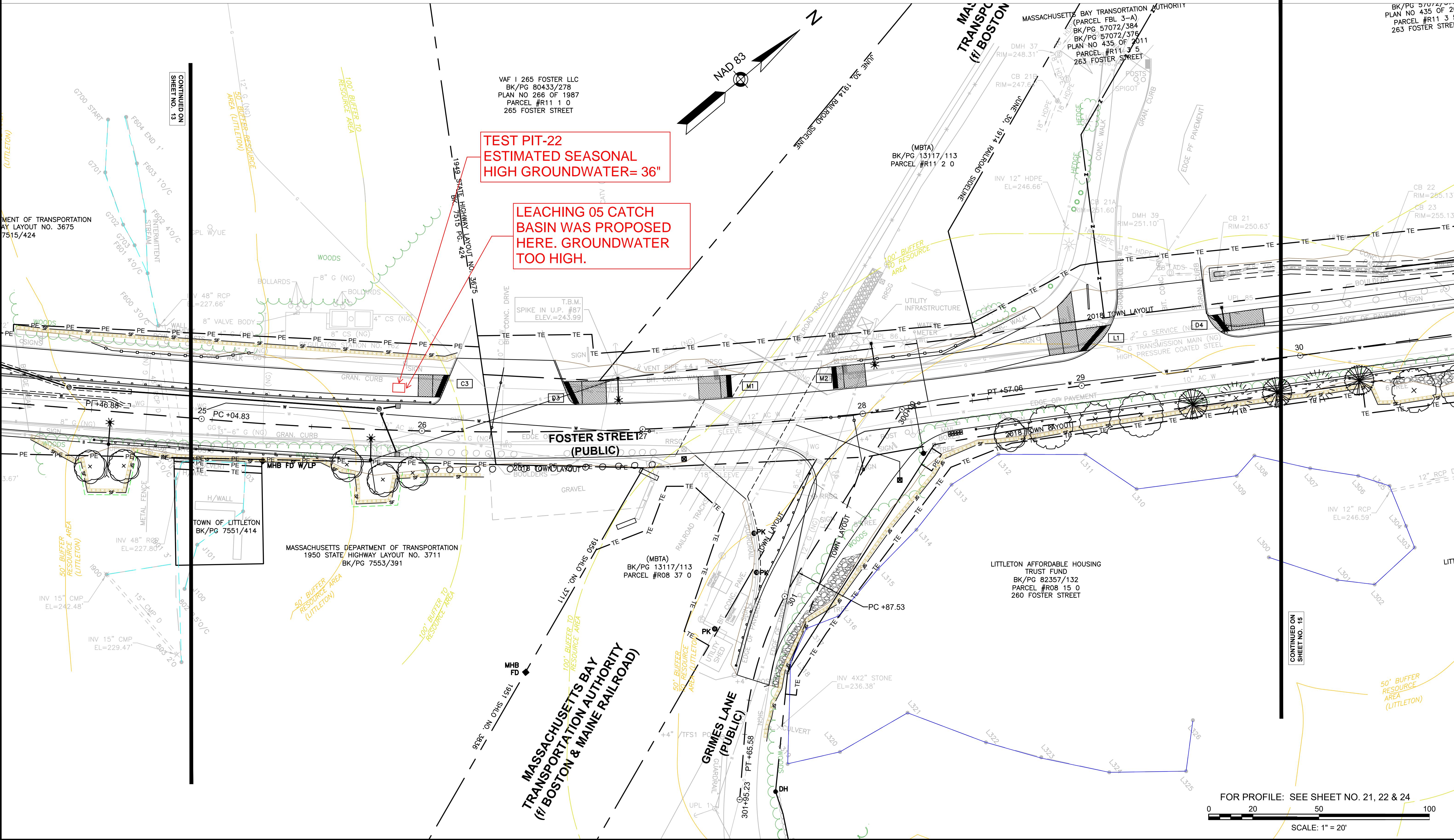
ENVIRONMENTAL PLANS LEGEND

LITTLETON RECONSTRUCTION OF FOSTER STREET

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP/CMQ/TAP-0033(037)X	E6	128
PROJECT FILE NO.		609054	

ENVIRONMENTAL PLANS

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ENVIRONMENTAL PLANS LEGEND

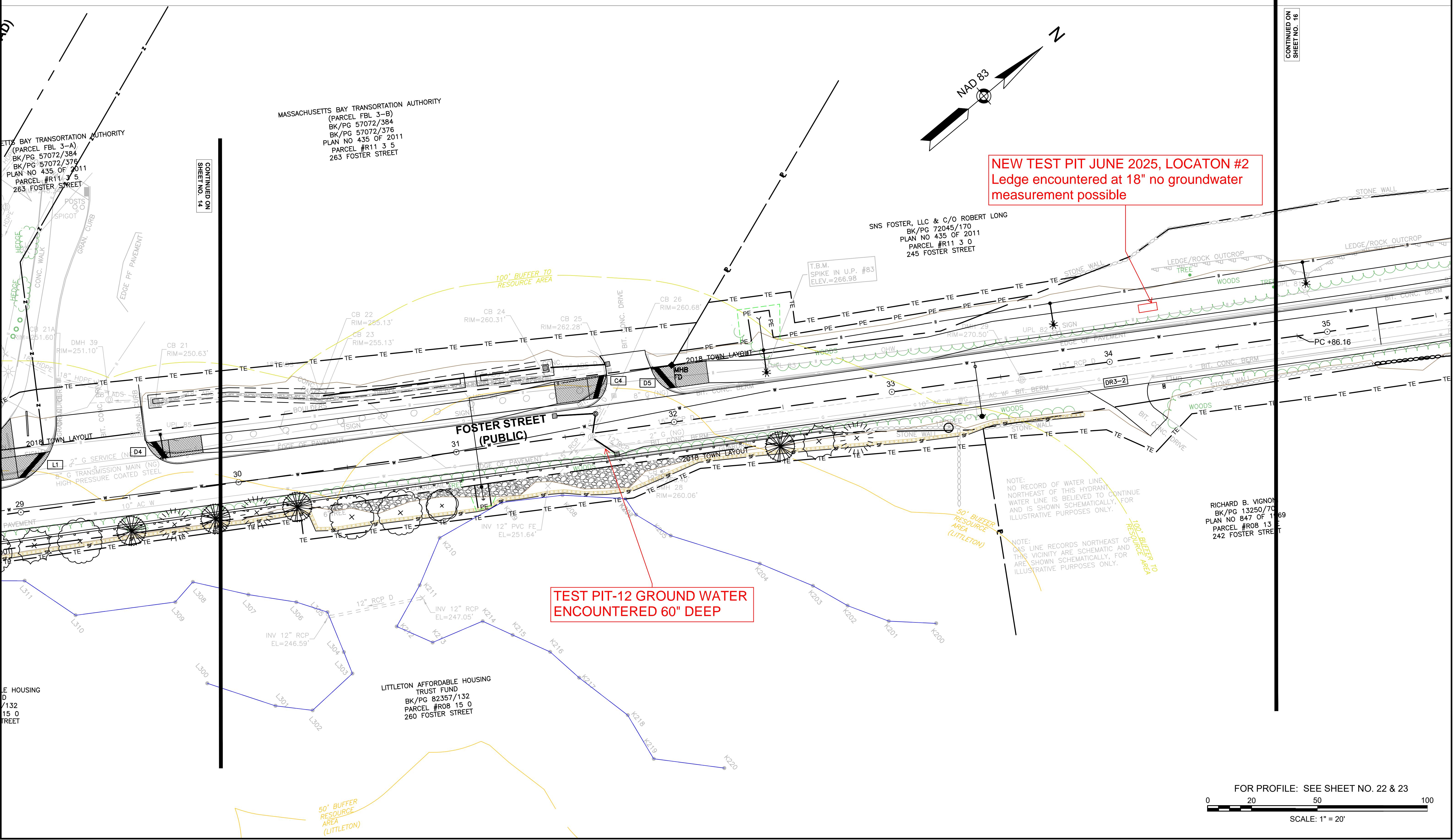
EXISTING TREE LINE	PROPOSED TREE LINE
PROPOSED TREE LINE	PROPOSED TREE LINE
BORDERING VEGETATED WETLANDS	PERM IMPACT TO BUFFER ZONE
BANK/LAND UNDER WATER	TEMP IMPACT TO BUFFER ZONE
100FT BUFFER from BVW or BANK	PERM IMPACT TO 50-FT NO DISTURB
50FT NO DISTURB LIMIT from BVW or BANK	TEMP IMPACT TO 50-FT NO DISTURB
EROSION CONTROLS	IMPERVIOUS REMOVED FROM 50-FT NO DISTURB
LIMIT OF WORK	IMPERVIOUS REMOVED FROM BUFFER ZONE

LITTLETON RECONSTRUCTION OF FOSTER STREET			
STATE	FED. AID PROJ. NO.	HEET NO.	TOTAL SHEETS
MA	STP/CMQ/TAP-003(037)X	E7	128
PROJECT FILE NO. 609054			

ENVIRONMENTAL PLANS

Plotted on 5-Jun-2025 4:17 PM

20170044A21 HPN01 NOI SET TEST PITS.DWG



ENVIRONMENTAL PLANS LEGEND

Legend for site plan symbols and lines:

- EXISTING TREE LINE: Green wavy line
- PROPOSED TREE LINE: Green dashed line
- BORDERING VEGETATED WETLANDS: Blue solid line
- BANK/LAND UNDER WATER: Cyan dashed line
- 100FT BUFFER from BVW or BANK: Yellow solid line
- 50FT NO DISTURB LIMIT from BVW or BANK: Orange solid line
- EROSION CONTROLS: Gold rectangular blocks
- LIMIT OF WORK: Brown solid line

	PERM IMPACT TO BUFFER ZONE
	TEMP IMPACT TO BUFFER ZONE
	PERM IMPACT TO 50-FT NO DISTURB
	TEMP IMPACT TO 50-FT NO DISTURB
	IMPERVIOUS REMOVED FROM 50-FT NO DISTURB
	IMPERVIOUS REMOVED FROM BUFFER ZONE

LITTLETON
RECONSTRUCTION OF FOSTER STREET

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP/CMQ/TAP-0033(037)X	E8	128
PROJECT FILE NO. 609054			

ENVIRONMENTAL PLANS

20170044A21_HPN01 NOI SET TEST PITs.DWG Plotted on 5-Jun-2025 4:17 PM

CONTINUED ON SHEET NO. 15

SNS FOSTER, LLC & C/O ROBERT LONG
BK/PG 72045/170
PLAN NO 435 OF 2011
PARCEL #R11 3 0
245 FOSTER STREET

END OF PROJECT
PROJ. NO. 609054
STA. 39+36.15
N 3015278.1762
E 656130.7752

NOTE:
C.B. 34 IS A DOUBLE-SIZED STRUCTURE.

NEW TEST PIT JUNE 2025, LOCATION #3
Ledge encountered at 36" no groundwater measurement possible

TEST PIT-33 ESTIMATED SEASONAL HIGH GROUNDWATER= 108"

NEW FULL-SIZE LEACHING CATCH BASIN INSTALLED HERE

RICHARD B. VIGNONI
BK/PG 13250/70
PLAN NO 847 OF 1969
PARCEL #R08 13 E
242 FOSTER STREET

QUINN E. & JENNIFER A. CANFIELD
BK/PG 55588/290
PLAN NO 188 OF 1971
PARCEL #R08 13 D
238 FOSTER STREET

FOR PROFILE: SEE SHEET NO. 23

SCALE: 1" = 20'

ALTERNATIVES ANALYSIS FOR REPLACEMENT OF 4 LEACHING CATCH BASINS

Alternatives Analysis for Replacement of Leaching Catch Basins

Request for Amendment to Order of Conditions for DEP#204-0991

The Reconstruction of Foster Street, MassDOT Project # 609054

July 2, 2025

The following narrative describes the stormwater infiltration measures that have been evaluated to replace the stormwater infiltration storage volume of the 4 MassDOT Standard leaching catch basins that could not be installed in within the project limits due to high estimated seasonal groundwater throughout the project corridor.

The project corridor along Foster Street, where the 0.6 acres of impervious area are proposed to construct a 10ft wide shared-use path along the roadside, is constrained by the following factors.

- Right of Way: The town owned public road layout of Foster Street is varies between 40 feet and 70 feet wide. The cross section of Foster Street has been minimized for lane width and shared-use path width totaling 40ft wide. This means that in some areas in the corridor, there is no available right of way outside of the road and path cross section limits. There is approx. 15 feet of width at each roadside in areas where the road layout is 70 feet wide. However, there are often other physical constraints that prohibit the use of this area.
- Trees: Foster Street is lined with trees and forest on each side of the road shoulder. Incurring additional tree takings are a constraint when considering widening the project limits to include stormwater control measures such as swales or basins.
- Groundwater Depth: The project has performed 10 test pits by a soil evaluator throughout the corridor. The results show groundwater shallower than 4feet in most locations tested. Only one location had the required groundwater depth, greater than 8.5' to install a MassDOT standard leaching catch basin. See attached test pit results.
- Constructability: The road corridor has existing shoulder slopes in many locations that have steep, 4:1, slopes or greater pitched toward or away from the edge of road. This means that significant cut and/or fill would be required to install stormwater infiltration measures by the roadside that require significant width and mildly sloped grading. Significant cut and fill increase the likelihood of adjacent private property impacts as the project limits must be tied back into existing ground.
- Ledge: The project has significant areas of shallow ledge approx. 4'-5' from the surface. This constraint makes the constructability of deep infiltration systems impractical where the ledge exists.

The alternatives analysis considered the following six stormwater infiltration countermeasures, mostly from MassDOT's stormwater design guide¹, to offset the 4 leaching catch basins that could not be installed.

- Permeable pavement
- Pavement disconnection
- Infiltration basin
- Linear infiltration or swale
- Subsurface infiltration system
- Crushed stone diaphragm trench

The following graphic summarizes the results of the alternative analysis by displaying a plus sign where an alternative provided an advantage and a negative sign where an alternative presented a drawback.

	Permeable Pavement	Pavement Disconnection	Infiltration Basin	Linear Infiltration or Swale	Subsurface Infiltration System	Crushed Stone Diaphragm Trench
Right-of-Way	+	-	-	+	+	+
Groundwater Elev. Requirement	-	+	-	-	-	+
Maintenance	-	+	-	+	-	-
Tree Impacts	+	-	-	-	+	+
Constructability	-	-	-	-	-	+

The crushed stone diaphragm trench is the proposed alternative to offset the stormwater infiltration storage that could not be installed via the 4 leaching MassDOT standard catch basins. Calculations and plans are included detailing the proposed crushed stone diaphragm trench. Next, each alternative is described in more detail with respect to their respective strengths and weaknesses.

¹ Stormwater Design Guide. MassDOT. 2023 Edition. <<https://www.mass.gov/doc/stormwater-design-guide/download>>

1. Permeable Pavement

Porous pavement is a pavement system designed to allow stormwater to infiltrate through a permeable surface, base, and sub-base. Porous pavement systems provide water quality treatment through filtration and infiltration mechanisms. MassDOT's preferred design for porous pavement is as an exfiltrating system with an underdrain within the reservoir layer to prevent surcharge conditions. F&O considered the possibility of converting an equivalent volume of some portion of the shared-use path to permeable pavement to offset the reduction in leaching catch basins. However, factors such as groundwater depth requirements, maintenance, and constructability resulted in the infeasibility of this alternative.

Table 1: Permeable Pavement Alternative Evaluation for the Foster Street Project

Right-of-Way	Groundwater Elev.	Maintenance	Tree Impacts	Constructability
<ul style="list-style-type: none"> The project ROW would accommodate this alternative if, for example, the 10ft shared-use path was constructed from permeable pavement in the proposed alignment. The road surfaces within the project limit cannot be constructed from permeable pavement per MassDOT standards due to sand treatment during winter and past MassDOT experience with freeze-thaw cycle damage to permeable road pavement. 	<ul style="list-style-type: none"> The minimum depth equals: 4" Choker course 8" Filter course 3" Filter blanket 4" Reservoir Course Total Depth= 19" The minimum depth of est. seasonal high groundwater must be 2ft below the bottom of reservoir course of crushed stone. Using the minimum depth above of 19", the minimum depth of groundwater must be 43". This required minimum depth limits the locations in the project area where permeable pavement is feasible for the shared-use path. 	<ul style="list-style-type: none"> Not for use in areas that receive sand treatment during winter Periodic cleaning or vacuuming of the surface is required to remove particulates. 	<ul style="list-style-type: none"> No Tree impacts anticipated if the shared-use path were constructed from permeable pavement within the project limits. 	<ul style="list-style-type: none"> Not well suited for use where underground utilities are beneath the pavement. This limits the locations in the project area where permeable pavement is feasible for the shared-use path. Due to the depth of construction required, may prove challenging to construct given ledge in project area at 4ft depth.

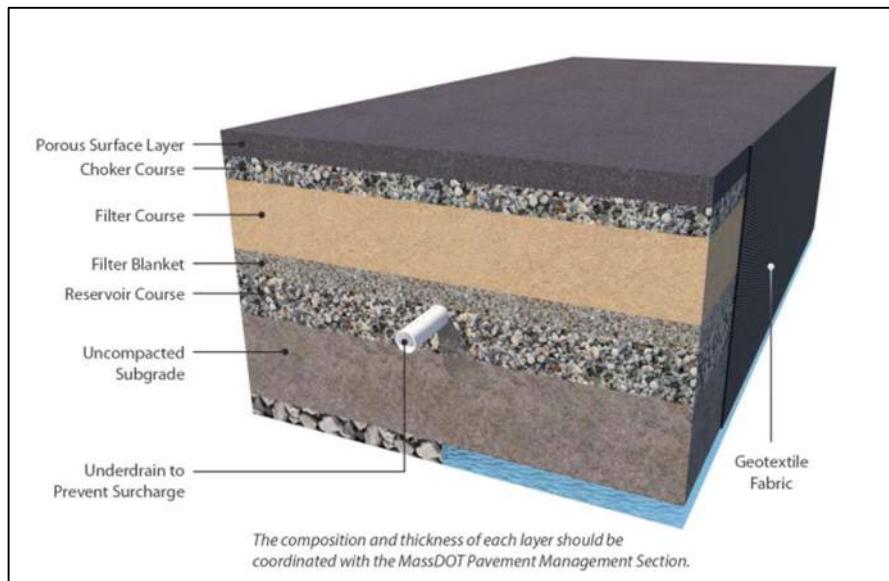


Figure 1: Example of Permeable Pavement Cross Section. Source: MassDOT Stormwater Design Guide (2023 Ed.)

2 Pavement Disconnection

Pavement disconnection is a low impact development measure that is also considered an infiltration SCM. This design approach involves directing runoff from impervious areas to vegetated upland areas, which may be intentional or incidental, engineered or natural. Approximately 30 percent of the length of the proposed 10ft wide shared-use path is already designed to be sloped away from the road, Foster Street. The sections of shared-use path that are sloped away from the road drain by sheet flow to vegetated areas. F&O considered the possibility of converting additional sections of the shared-use path to slope toward vegetated areas. However, factors such as ROW constraints, tree impacts, and constructability resulted in the infeasibility of this alternative.

Table 2: Pavement Disconnection Alternative Evaluation for the Foster Street Project

Right-of-Way	Groundwater Elev.	Maintenance	Tree Impacts	Constructability
<ul style="list-style-type: none"> A width of vegetative filter strip equal in width to the contributing impervious area is needed. For the 10ft wide shared-use path, a 10ft wide vegetative filter strip with a slope less than or equal to 5% downgradient from the shared-use path is required. The project ROW limits and gradients at adjacent private properties limit the locations where pavement disconnection is feasible. 	<ul style="list-style-type: none"> No minimum est. high seasonal high groundwater is given for this alternative. Although 24-inch minimum separation from groundwater is generally recommended for groundwater recharge. 	<ul style="list-style-type: none"> General maintenance of vegetative area to maintain sheet flow path of storm runoff. 	<ul style="list-style-type: none"> In some areas of the project limits, creation of a 10ft wide vegetative strip adjacent to the share-use path would impact existing trees. This would occur where cutting of filling of earth would be required to meet the 5% slope requirement. 	<ul style="list-style-type: none"> This alternative would result in the resetting of additional stone wall adjacent to the road side. The 5% maximum gradient requirement for the 10ft wide vegetative strip adjacent to the shared-use path would result in additional cut/fill where the slope limits of the project must meet existing ground elevation. In various locations, the amount of cut/fill required is impractical due to steep embankment slopes at the roadside.

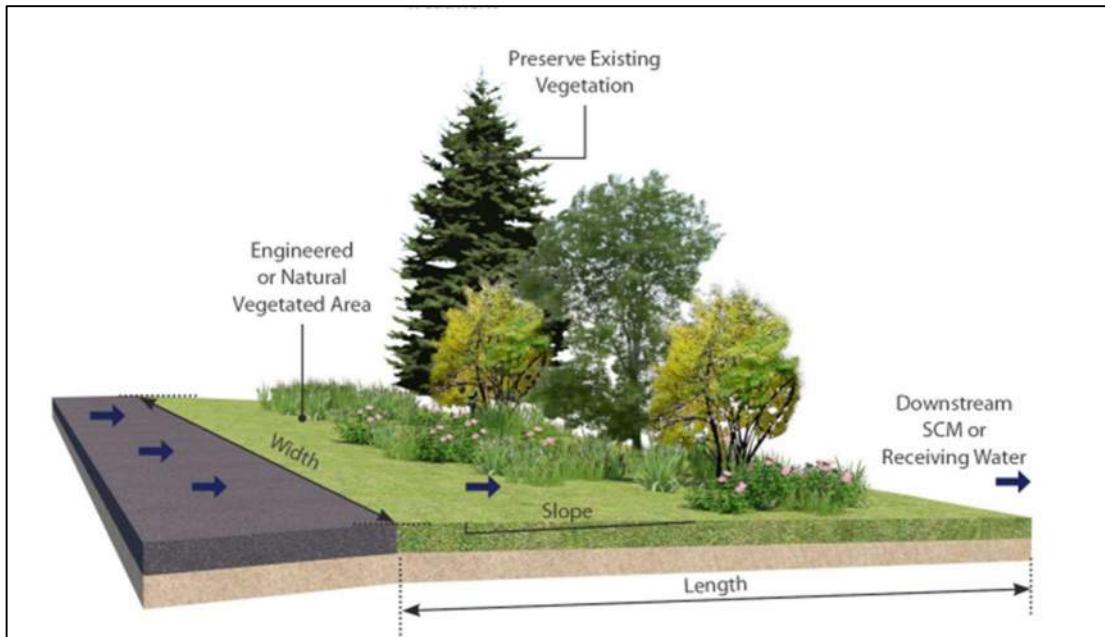


Figure 2: Example of Pavement Disconnection Source: MassDOT Stormwater Design Guide (2023 Ed.)

3 Infiltration Basin

Infiltration basins are designed to provide water quality treatment through storage using outlet control and/or behind check dams and infiltrate runoff to groundwater. The project included a conceptual infiltration basin at 25% design stage in 2019. The basin was proposed on MBTA property at 263 Foster Street, the only publicly owned and relatively flat lawn area adjacent to the project corridor. ROW easement complexities and insurance requirements on MBTA property prohibited this infiltration basin from moving forward past 25% design. Given the constrained width of the project ROW, potential tree impacts at the roadside, and groundwater depth requirements, this alternative has been found to be infeasible.

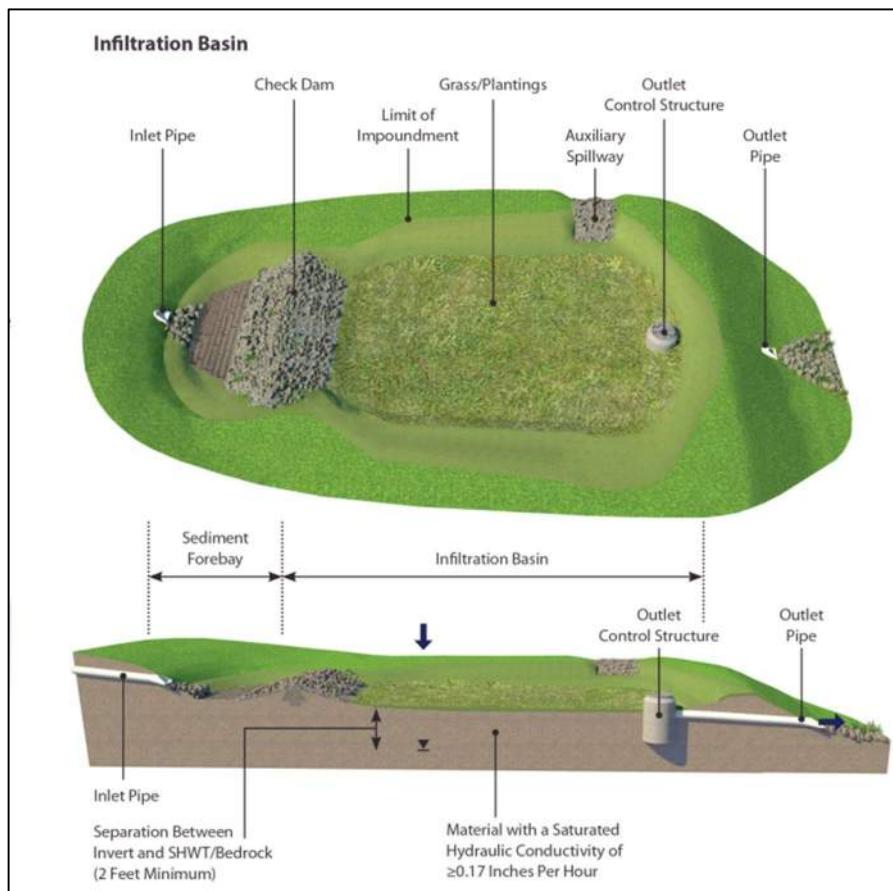


Figure 3: Example of an Infiltration Basin. Source MassDOT Stormwater Guide (2023)

Table 3: Infiltration Basin Alternative Evaluation for the Foster Street Project

Right-of-Way	Groundwater Elev.	Maintenance	Tree Impacts	Constructability
<ul style="list-style-type: none"> The project ROW limits and gradients at adjacent private properties render the construction of an infiltration basin impractical. Basins are recommended to have a ground slope of 1% maximum. 	<ul style="list-style-type: none"> An infiltration basin is typically set at a ground elevation to receive flow from an outlet pipe of a closed drainage system. The minimum depth of burial of closed drainage system pipe is typically 2'-3' from elevation of ground cover to bottom of pipe invert. 24-inch minimum separation from ground surface of the infiltration basin to groundwater is required for groundwater recharge. Pipe invert depth for inlet pipe plus required separation results in min. ground water depths of 4'-5'. This limits the locations in the project area where this alternative is feasible. 	<ul style="list-style-type: none"> Maintenance of infiltration basins generally consists of removing the build up of particulates and debris in the basin and maintaining clearance of inlet and outlet pipes. 	<ul style="list-style-type: none"> Existing trees lining Foster Street and the proposed shared-use path would result in more tree takings if an infiltration basin could be implemented. 	<ul style="list-style-type: none"> This alternative would result in removal of stone wall adjacent to the roadside. An infiltration basin would require significant regrading of earth adjacent to the roadside to maintain a 1% ground slope within the infiltration basin. Tying the constructed basin back to existing ground at adjacent private property adjacent to the road is impractical in many locations. Due to the depth of construction required, may prove challenging to construct given ledge in project area at 4ft depth.

4 Linear Infiltration or Swale

Infiltration linear practices or swales are essentially a series of infiltration basins in a linear configuration that use the same treatment mechanisms as basins to improve water quality and infiltration. F&O considered the possibility of converting additional areas at the edge of road or edge of shared-use path to infiltrative swales. However, factors such as ROW constraints, tree impacts, and required depth of groundwater resulted in this alternative to be less feasible than alternative 6, crushed stone diaphragm trench.

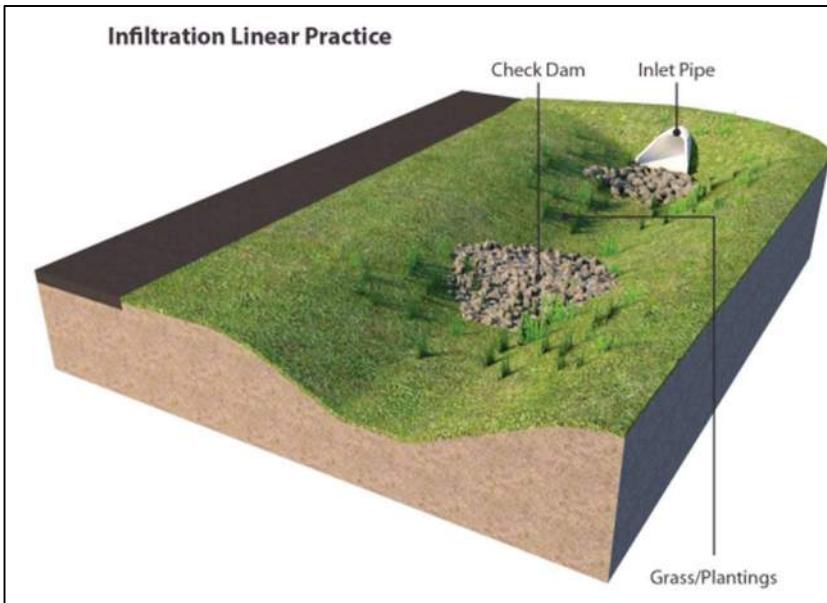


Figure 4: Example of Linear Infiltration Practice. Source: MassDOT Stormwater Guide (2023)

Table 4: Linear Infiltration or Swale Alternative Evaluation for the Foster Street Project

Right-of-Way	Groundwater Elev.	Maintenance	Tree Impacts	Constructability
<ul style="list-style-type: none"> The project ROW limits at adjacent private properties limit the locations where linear swales are constructable in the project limits at the roadside or at the edge of the share-use path. 	<ul style="list-style-type: none"> A linear swale is typically set at a ground elevation to receive flow from an outlet pipe of a closed drainage system. The minimum depth of burial of closed drainage system pipe is typically 2'-3' from elevation of ground cover to bottom of pipe invert. 24-inch minimum separation from the bottom of the swale to groundwater is required for groundwater recharge. Pipe invert depth for inlet pipe plus required separation results in min. ground water depths of 4'-5'. This limits the locations in the project area where this alternative is feasible. 	<ul style="list-style-type: none"> Low maintenance. Maintenance of infiltration swales generally consists of removing the buildup of particulates and debris in the swale and maintaining clearance of inlet and outlet pipes. 	<ul style="list-style-type: none"> Existing trees lining Foster Street and the proposed shared-use path would result in more tree takings if slope limits are increased to construct swales. 	<ul style="list-style-type: none"> This alternative would result in additional relocating of stone wall adjacent to the roadside. The required width of linear swales at the roadside or the back of the shared-use path would be at least 3'-4". In various locations this width is not available due to existing obstructions, trees, or existing embankments. Linear infiltration swales are not recommended where underground utilities exist. Due to the depth of construction required, may prove challenging to construct given ledge in project area at 4ft depth.

5 Subsurface Infiltration System

Subsurface infiltration systems are underground systems designed to detain stormwater and release it to groundwater through infiltration. F&O considered the possibility of converting segments underneath the proposed shared-use path to a subsurface infiltration system. However, the required depth of groundwater, approx. 4.5ft, resulted in this alternative to be infeasible. The one location where estimated seasonal high groundwater was found to be deep enough to support a subsurface infiltration system was on the opposite side of the road from the shared-use path at STA 35+84. The shared-use path near this location is located on a side of the road with extremely shallow ledge. Two test pits in this area struck shallow ledge at less than 24 inches from the proposed surface to the shared use path. The shared-use path cannot be moved to the opposite side of the road at this location.

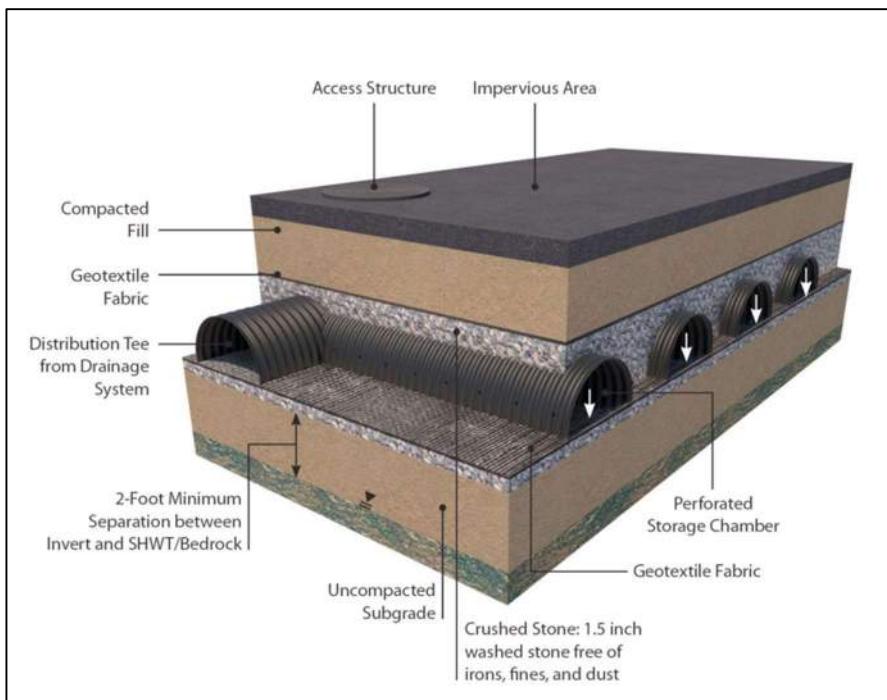


Figure 5: Example of Subsurface Infiltration System. Source: MassDOT Stormwater Guide (2023)

Table 5: Subsurface Infiltration Systems Evaluation for the Foster Street Project

Right-of-Way	Groundwater Elev.	Maintenance	Tree Impacts	Constructability
<ul style="list-style-type: none"> The narrow project corridor would require any subsurface infiltration system to be linear in form underneath the shared-use path. Subsurface infiltration systems are not recommended by MassDOT directly beneath public roads. 	<ul style="list-style-type: none"> The pavement structure and subbase of the shared use path has a depth of 1ft. The smallest subsurface infiltration systems typically have a 1 ft tall plastic structure with required crushed stone cover of at least 6in. Ground water depths are typically recommended to be 2ft below the infiltration system. The sum of the shared-use path pavement depth (1ft), the subsurface infiltration system (1.5ft), and separation from groundwater (2ft) total 4.5ft or 54in. This is a practical minimum for seasonal high groundwater elevation for this alternative. 	<ul style="list-style-type: none"> Subsurface infiltration systems include one or more cleanout ports at ground level. Owners must inspect and clean the systems through the port periodically. 	<ul style="list-style-type: none"> N/A if located beneath the propose shared-use path. 	<ul style="list-style-type: none"> This alternative is not feasible where underground utilities, such as gas mains, are located beneath the shared-use path. Due to the depth of construction required, may prove challenging to construct given ledge in project area at 4ft depth.

6 Crushed Stone Diaphragm Trench

A stone diaphragm trench is a shallow trench filled with crushed stone that can act as either a pretreatment device and/or to promote infiltration. The trench is wrapped in geotextile fabric for separation and to prevent particulates from entering the trench. The trench includes a 3" top layer of crushed stone outside of the geotextile fabric to provide treatment. The 3-inch layer requires periodic replacement or replenishment for maintenance. Because the stone diaphragm sits at surface level, it does not require as much excavation or require as deep an elevation of groundwater depth compared to other alternatives. When the crushed stone diaphragm becomes saturated, stormwater runoff will either sheet flow over the trench to a down sloped vegetated area or return to the closed drainage system on Foster Street.

This alternative has been found to be the most feasible option to replace the stormwater storage volume of the 4 leaching catch basins that could not be installed. Calculations are attached to this submittal showing an equivalency between the storage volume of a proposed 100ft crushed stone diaphragm and the 4 leaching catch basins that could not be installed in the project limits. The location of the proposed crushed stone diaphragm is between STA 34+21 and STA 35+70 on the right-side edge of pavement of Foster Street.

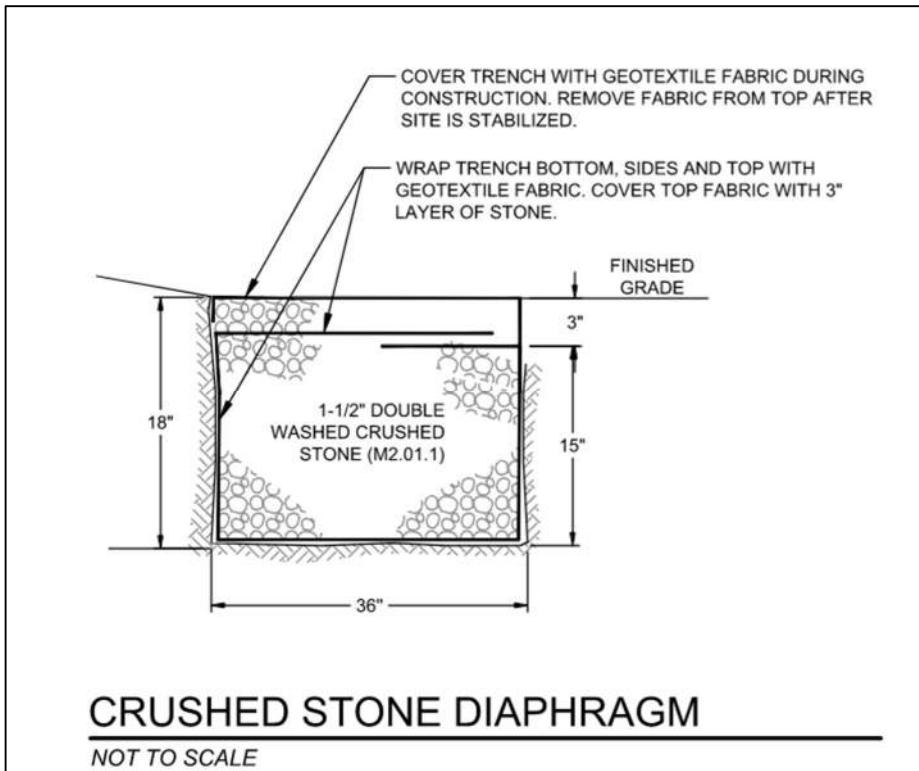


Figure 6: Example of a Crushed Stone Diaphragm.

Table 6: Crushed Stone Diaphragm Trench Evaluation for the Foster Street Project

Right-of-Way	Groundwater Elev.	Maintenance	Tree Impacts	Constructability
<ul style="list-style-type: none"> This alternative is feasible within the project ROW due the narrow 3ft wide linear footprint. 	<ul style="list-style-type: none"> This alternative sits at the ground surface and has a proposed depth of 1.5ft. It is recommended that seasonal high groundwater is located at least 2ft below the bottom of trench. The sum of the depth of trench (1.5ft) and the required separation (2ft) from ground water results in a minimum groundwater depth of 3.5ft or 42in. 	<ul style="list-style-type: none"> The top 3" of crushed stone must be replenished and replaced periodically with cleaned crushed stone to maintain depth and remove debris and particulates. 	<ul style="list-style-type: none"> N/A or minor due the narrow 3ft width from the edge of pavement. 	<ul style="list-style-type: none"> Low complexity/ Low conflict to construct. This alternative is not feasible where underground utilities, such as gas mains, are located at the roadside.

**PLAN SET MARKUP AND DETAIL OF
CRUSHED STONE DIAPHRAGM
TRENCH**

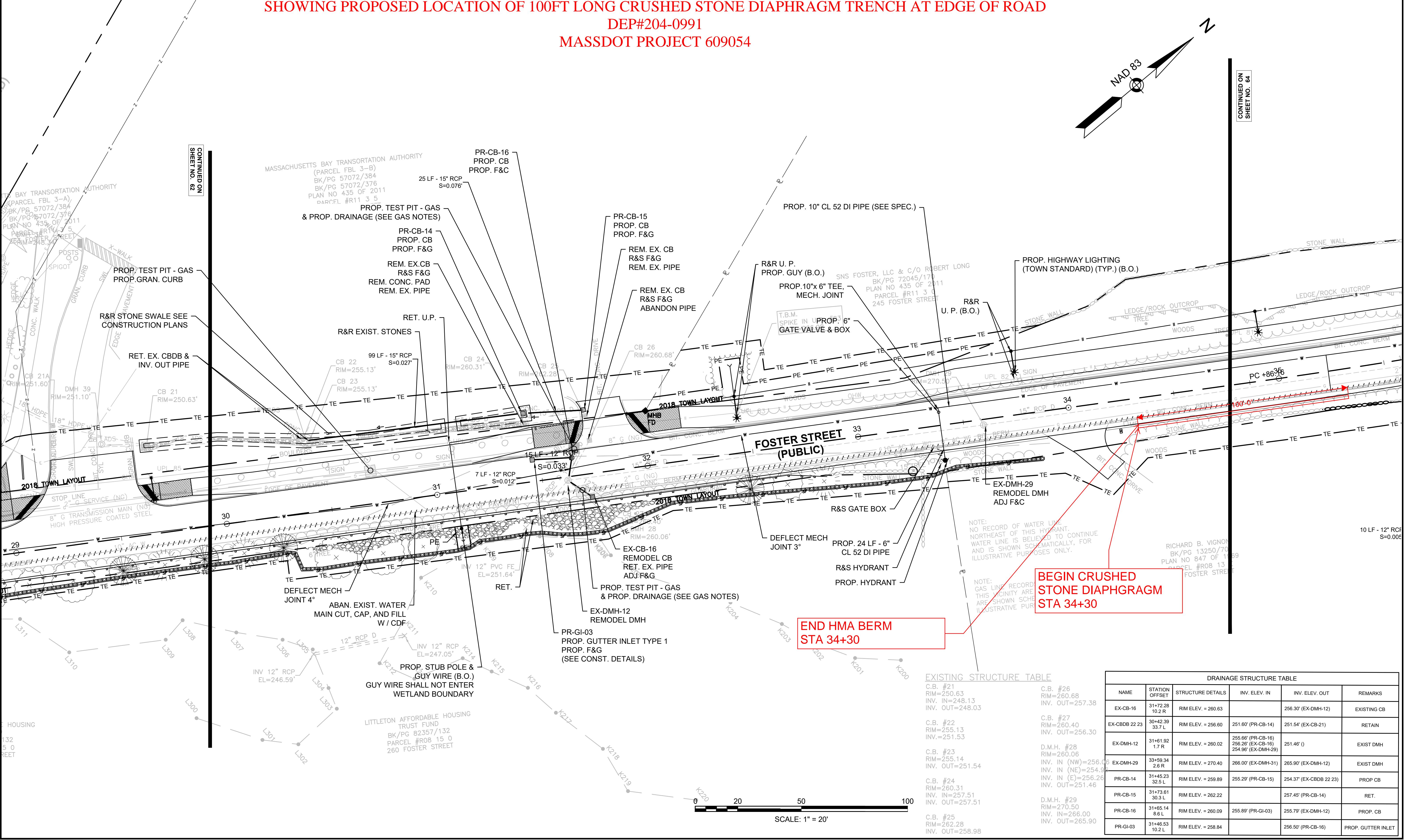
NOTES:
1. SEE SHEET 57 FOR GENERAL UTILITY, ELECTRICAL AND TREE TRIMMING NOTES
2. * - CONTRACTOR TO MATCH EXISTING PIPE INVERT. INVERT GIVEN IS APPROXIMATE

GAS NOTES:
1. NOTIFY NATIONAL GRID DAMAGE PREVENTION, MEGHAN KELLEY, AT(339) 203-0490 TWO WEEKS PRIOR TO EXCAVATING NEAR REGULATOR STATION OR GAS MAIN. HIGH PRESSURE GAS MAINS ARE PRESENT IN PROJECT AREA.
2. TO REPORT ANY DAMAGE TO A GAS LINE CALL NATIONAL GRID'S EMERGENCY GAS LEAKS NUMBER AT 1-800-233-5325 IMMEDIATELY.
3. ALL TEST PITS PERFORMED NEAR GAS MAIN SHALL BE PERFORMED WITH A VACUUM TRUCK SEE SPECIAL PROVISION FOR ITEM 141.101

LITTLETON
RECONSTRUCTION OF FOSTER STREET
STATE FED. AID PROJ. NO. SHEET NO. TOTAL SHEETS
MA STP/CMQ/TAP-003(037)X 63 128
PROJECT FILE NO. 609054
DRAINAGE & UTILITY PLANS

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FOSTER STREET PLAN MARKUPS
SHOWING PROPOSED LOCATION OF 100FT LONG CRUSHED STONE DIAPHRAGM TRENCH AT EDGE OF ROAD
DEP#204-0991
MASSDOT PROJECT 609054

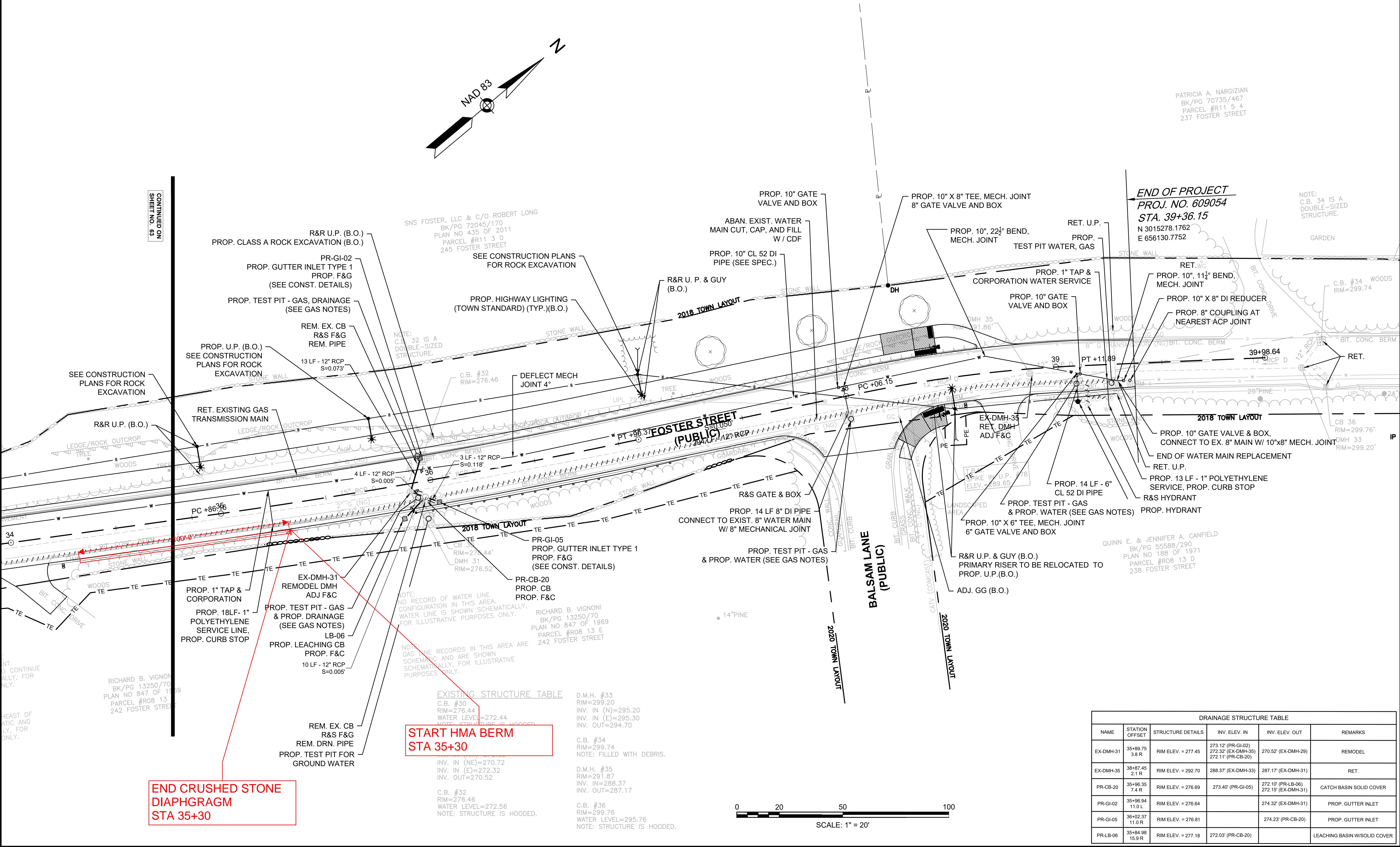


NOTES:
1. SEE SHEET 57 FOR GENERAL UTILITY, ELECTRICAL AND TREE TRIMMING NOTES
2. * - CONTRACTOR TO MATCH EXISTING PIPE INVERT. INVERT GIVEN IS APPROXIMATE

GAS NOTES:
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LITTLETON
RECONSTRUCTION OF FOSTER STREET
STATE FED. AID PROJ. NO. SHEET NO. TOTAL SHEETS
MA STP/CMQ/TAP-0033(037)X 64 128
PROJECT FILE NO. 609054
DRAINAGE & UTILITY PLANS

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DANIEL SWEENEY (CLASS A TRUSTEE)
ARTURO J. GUTIEREEZ, JOHN A.
CATALDO (CLASS B TRUSTEES)
TAYLOR STREET, LITTLETON TRUST
BK/PG 32096/213
PARCEL #R10 6 0
TAYLOR STREET

BEGINNING OF PROJECT
PROJ. NO. 609054
STA 0+00.00 
N3012786.8847
E653296.9381

DANIEL SWEENEY (CLASS A TRUSTEE)
ARTURO J. GUTIERREZ, JOHN A.
CATALDO (CLASS B TRUSTEES)
TAYLOR STREET, LITTLETON TRUST

LIMIT OF WORKING
STA 205+00.00
N3012961.1405
E653254.4720

ENVIRONMENTAL PLANS LEGE

PROPOSED TREE LINE

BORDERING VEGETATED WETLANDS

BANK/LAND UNDER WATER

100FT BUFFER from BVW or BANK

50FT NO DISTURB LIMIT from BVW or BANK

EROSION CONTROLS

LIMIT OF WORK

PERM IMPACT TO BUFFER ZONE

TEMP IMPACT TO BUFFER ZONE

PERM IMPACT TO 50-FT NO DISTURB

TEMP IMPACT TO 50-FT NO DISTURB

IMPERVIOUS REMOVED FROM 50-FT NO DISTURB

IMPERVIOUS REMOVED FROM BUFFER ZONE

2641-2651 SANTA ANNA AVENUE, LLC
BK/PG 68756/572
PLAN NO 1425 OF 1981
PARCEL #R10 2 1
305 FOSTER STREET

LINE OF PAVEMENT

— 9 —

PLANT LIST

KEY BOTANICAL NAME
TREES

JOHN K. GRADY, TRUSTEE OF THE
FOSTER/TAYLOR REALTY TRUST
BK/PG 25198/143
PLAN NO 228 OF 1992
PARCEL #R09 32 0
230 TAYLOR STREET

JOHN K. GRADY & DAVID B. RICE, TRUSTEES OF
CONCORD ASSOCIATES FOSTER STREET TRUST
BK/PG 14680/362
PLAN NO 1314 OF 1981
PARCEL #R09 33 0
300 FOSTER STREET

<u>COMMON NAME</u>	<u>HT.</u>	<u>QTY.</u>	<u>SIZE</u>
MAPLE-RED-'OCTOBER GLORY'	50	5	2"-2.5" CAL

FOR PROFILE: SEE SHEET NO. 17

PORT ROYALE: SEE STREET NO. 17

A horizontal scale bar with tick marks at 20, 50, and 100.

SCALE: 1" = 20'

Digitized by srujanika@gmail.com

For more information, contact the Office of the Vice President for Research and the Office of the Vice President for Student Affairs.

PLANTING LEGEND

PROPOSED TREE PLANTING

PLANT QUANTITY AND SPECIES

ENVIRONMENTAL PLANS LEGEND

EXISTING TREE LINE

PROPOSED TREE LINE

BORDERING VEGETATED WETLANDS

BANK/LAND UNDER WATER

100FT BUFFER from BVW or BANK

50FT NO DISTURB LIMIT from BVW or BANK

EROSION CONTROLS

LIMIT OF WORK

	PERM IMPACT TO BUFFER ZONE
	TEMP IMPACT TO BUFFER ZONE
	PERM IMPACT TO 50-FT NO DISTURB
	TEMP IMPACT TO 50-FT NO DISTURB
	IMPERVIOUS REMOVED FROM 50-FT NO DISTURB
	IMPERVIOUS REMOVED FROM BUFFER ZONE

LITTLETON

RECONSTRUCTION OF FOSTER STREET			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP/CMQ/TAP-0033(037)X	E2	128
PROJECT FILE NO.		609054	

ENVIRONMENTAL PLANS

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FOR PROFILE: SEE SHEET NO. 18

PROPERTY LINE SEE STREET NO. 15

0 20 50 100

SCALE: 1" = 20'

ENVIRONMENTAL PLANS LEGEND

- EXISTING TREE LINE
- PROPOSED TREE LINE
- BORDERING VEGETATED WETLANDS
- BANK/LAND UNDER WATER
- 100FT BUFFER from BVW or BANK
- 50FT NO DISTURB LIMIT from BVW or BANK
- EROSION CONTROLS
- LIMIT OF WORK

	PERM IMPACT TO BUFFER ZONE
	TEMP IMPACT TO BUFFER ZONE
	PERM IMPACT TO 50-FT NO DISTURB
	TEMP IMPACT TO 50-FT NO DISTURB
	IMPERVIOUS REMOVED FROM 50-FT NO DISTURB
	IMPERVIOUS REMOVED FROM BUFFER ZONE

LITTLETON RECONSTRUCTION OF FOSTER STREET

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEET
MA	STP/CMQ/TAP-0033(037)X	E3	128
PROJECT FILE NO.		609054	

ENVIRONMENTAL PLANS

ASIJA PROPERTIES,
BK/PG 70688/
PLAN NO 1425 OF 1981
PARCEL #R10 2 2
295 FOSTER STREET

JOHN K. GRADY & DAVID B. RICE, TRUSTEES
CONCORD ASSOCIATES FOSTER STREET TRUST
BK/PG 14680/362
PLAN NO 1314 OF 1981
PARCEL #R09 33 0
120 FOSTER STREET

FOR PROFILE: SEE SHEET NO. 19



20 50

SCALE: 1" = 20'

ENVIRONMENTAL PLANS LEGEND

EXISTING TREE LINE
PROPOSED TREE LINE
BORDERING VEGETATED WETLANDS
BANK/LAND UNDER WATER
100FT BUFFER from BVW or BANK
50FT NO DISTURB LIMIT from BVW or BANK
EROSION CONTROLS
LIMIT OF WORK

PERM IMPACT TO BUFFER ZONE
TEMP IMPACT TO BUFFER ZONE
PERM IMPACT TO 50-FT NO DISTURB
TEMP IMPACT TO 50-FT NO DISTURB
IMPERVIOUS REMOVED FROM 50-FT NO DISTURB
IMPERVIOUS REMOVED FROM BUFFER ZONE

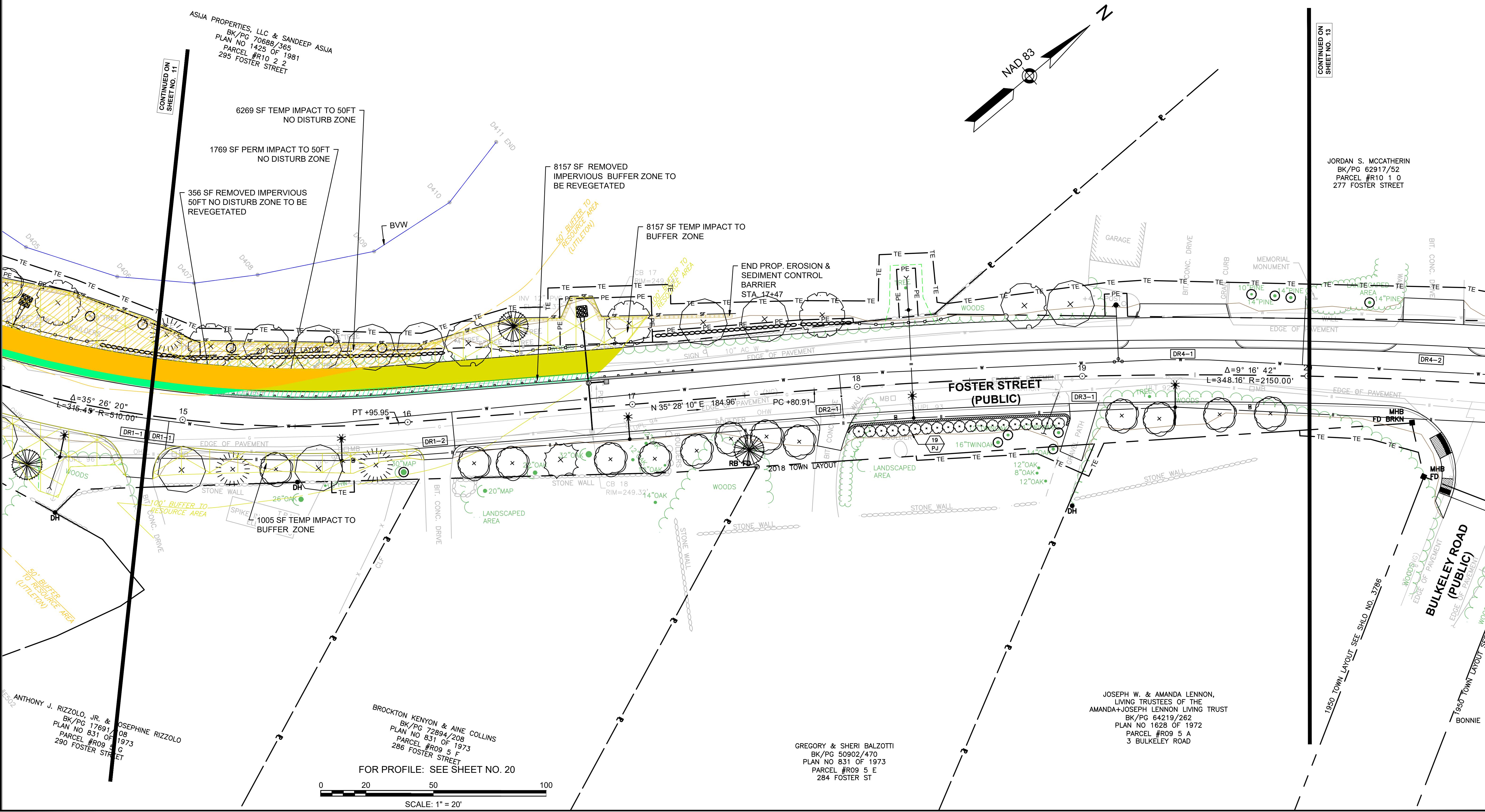
LITTLETON
RECONSTRUCTION OF FOSTER STREET

STATE	FED. AID PROJ. NO.	HEET NO.	TOTAL SHEETS
MA	STP/CMQ/TAP-0033(037)X	E4	128

PROJECT FILE NO. 609054

ENVIRONMENTAL PLANS

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ENVIRONMENTAL PLANS LEGEND

EXISTING TREE LINE

PROPOSED TREE LINE

BORDERING VEGETATED WETLANDS

BANK/LAND UNDER WATER

100FT BUFFER from BVW or BANK

50FT NO DISTURB LIMIT from BVW or BANK

EROSION CONTROLS

LIMIT OF WORK

	PERM IMPACT TO BUFFER ZONE
	TEMP IMPACT TO BUFFER ZONE
	PERM IMPACT TO 50-FT NO DISTURB
	TEMP IMPACT TO 50-FT NO DISTURB
	IMPERVIOUS REMOVED FROM 50-FT NO DISTURB
	IMPERVIOUS REMOVED FROM BUFFER ZONE

LITTLETON RECONSTRUCTION OF FOSTER STREET

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP/CMQ/TAP-0033(037)X	E5	128
PROJECT FILE NO.		609054	

ENVIRONMENTAL PLANS

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CONTINUED ON SHEET NO. 12

CONTINUED ON SHEET NO. 14

MASSACHUSETTS DEPARTMENT OF TRANSPORTATION
1949 STATE HIGHWAY LAYOUT NO. 3675
BK/PG 7515/424

MASSACHUSETTS DEPARTMENT OF TRANSPORTATION
1949 STATE HIGHWAY LAYOUT NO. 3675
BK/PG 7515/424

MASSACHUSETTS DEPARTMENT OF TRANSPORTATION
1949 STATE HIGHWAY LAYOUT NO. 3675
BK/PG 7515/424

MASSACHUSETTS DEPARTMENT OF TRANSPORTATION
1950 STATE HIGHWAY LAYOUT NO. 3711
BK/PG 7553/391

MASSACHUSETTS DEPARTMENT OF TRANSPORTATION
1950 STATE HIGHWAY LAYOUT NO. 3711
BK/PG 7553/391

MASSACHUSETTS DEPARTMENT OF TRANSPORTATION
1950 STATE HIGHWAY LAYOUT NO. 3711
BK/PG 7553/391

FOR PROFILE: SEE SHEET NO. 21

SCALE: 1" = 20'

ENVIRONMENTAL PLANS LEGEND

The diagram illustrates the layout of a proposed tree line, bordering vegetated wetlands, and erosion controls. It consists of several horizontal lines of different colors and patterns, each accompanied by a label:

- EXISTING TREE LINE: Represented by a series of green semi-circular arcs.
- PROPOSED TREE LINE: Represented by a dashed green line.
- BORDERING VEGETATED WETLANDS: Represented by a solid blue line.
- BANK/LAND UNDER WATER: Represented by a dashed cyan line.
- 100FT BUFFER from BVW or BANK: Represented by a solid yellow line.
- 50FT NO DISTURB LIMIT from BVW or B: Represented by a dashed orange line.
- EROSION CONTROLS: Represented by a series of small, square, outlined boxes containing dots.
- LIMIT OF WORK: Represented by a solid brown line.

	PERM IMPACT TO BUFFER ZONE
	TEMP IMPACT TO BUFFER ZONE
	PERM IMPACT TO 50-FT NO DISTURB
	TEMP IMPACT TO 50-FT NO DISTURB
	IMPERVIOUS REMOVED FROM 50-FT NO DISTURB
	IMPERVIOUS REMOVED FROM BUFFER ZONE

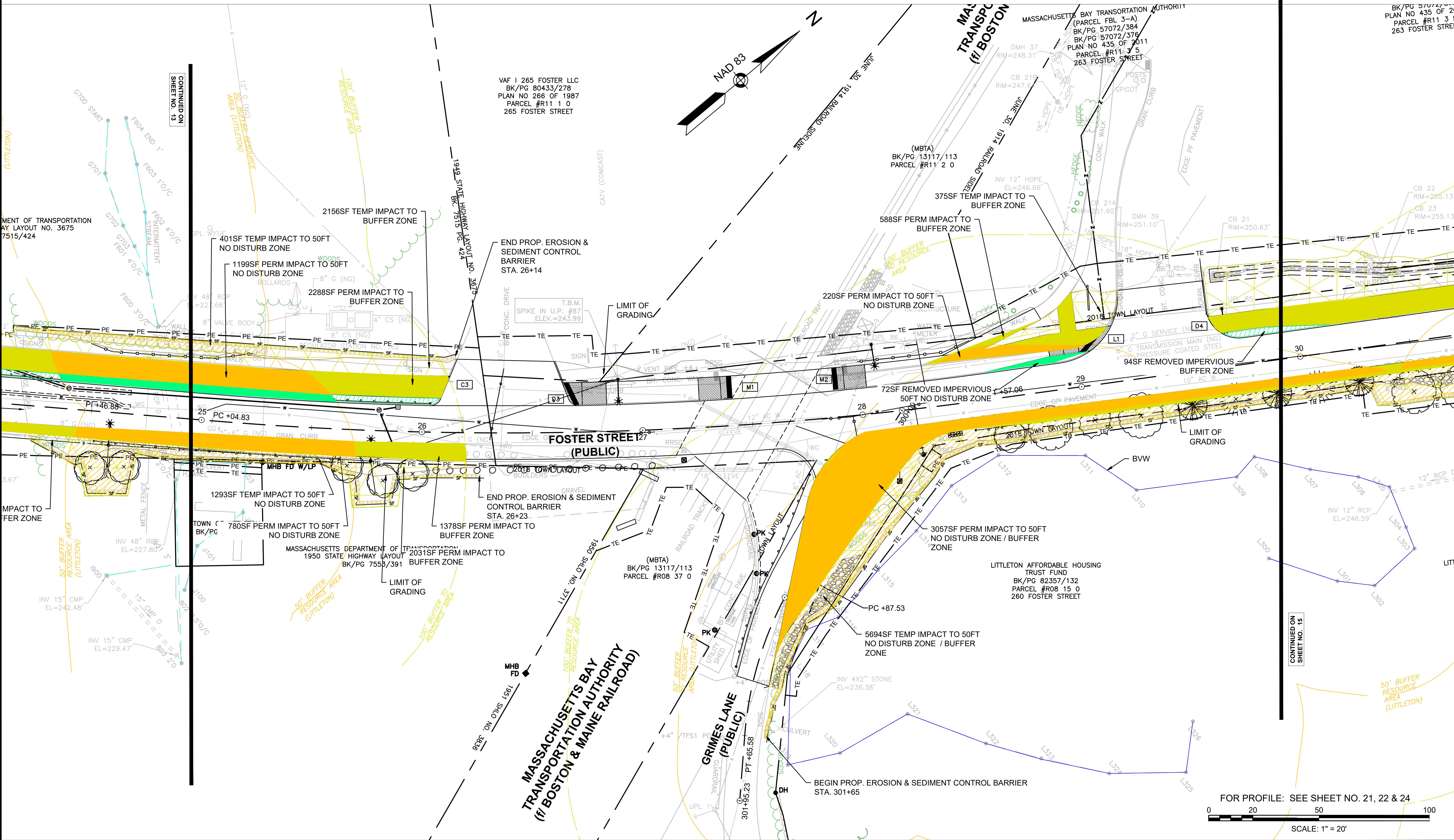
LITTLETON

RECONSTRUCTION OF FOSTER STREET

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP/CMQ/TAP-0033(037)X	E6	128
PROJECT FILE NO.		609054	

ENVIRONMENTAL PLANS

20170041121 HRN01 NO1 SET DWG
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ENVIRONMENTAL PLANS LEGEND

Legend for the streambank restoration project:

- EXISTING TREE LINE: Green wavy line
- PROPOSED TREE LINE: Green dashed line
- BORDERING VEGETATED WETLANDS: Blue solid line
- BANK/LAND UNDER WATER: Cyan dashed line
- 100FT BUFFER from BVW or BANK: Yellow solid line
- 50FT NO DISTURB LIMIT from BVW or B: Yellow dashed line
- EROSION CONTROLS: Brown rectangular boxes
- LIMIT OF WORK: Brown solid line

- PERM IMPACT TO BUFFER ZONE
- TEMP IMPACT TO BUFFER ZONE
- PERM IMPACT TO 50-FT NO DISTURB
- TEMP IMPACT TO 50-FT NO DISTURB
- IMPERVIOUS REMOVED FROM 50-FT NO DISTURB
- IMPERVIOUS REMOVED FROM BUFFER ZONE

LITTLETON RECONSTRUCTION OF FOSTER STREET

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP/CMQ/TAP-0033(037)X	E7	128
PROJECT FILE NO.		609054	

ENVIRONMENTAL PLANS

ENVIRONMENTAL PLANS LEGEND

~~~~~	EXISTING TREE LINE
-----	PROPOSED TREE LINE
-----	BORDERING VEGETATED WETLANDS
-----	BANK/LAND UNDER WATER
-----	100FT BUFFER from BVW or BANK
-----	50FT NO DISTURB LIMIT from BVW or BANK
-----	EROSION CONTROLS
-----	LIMIT OF WORK

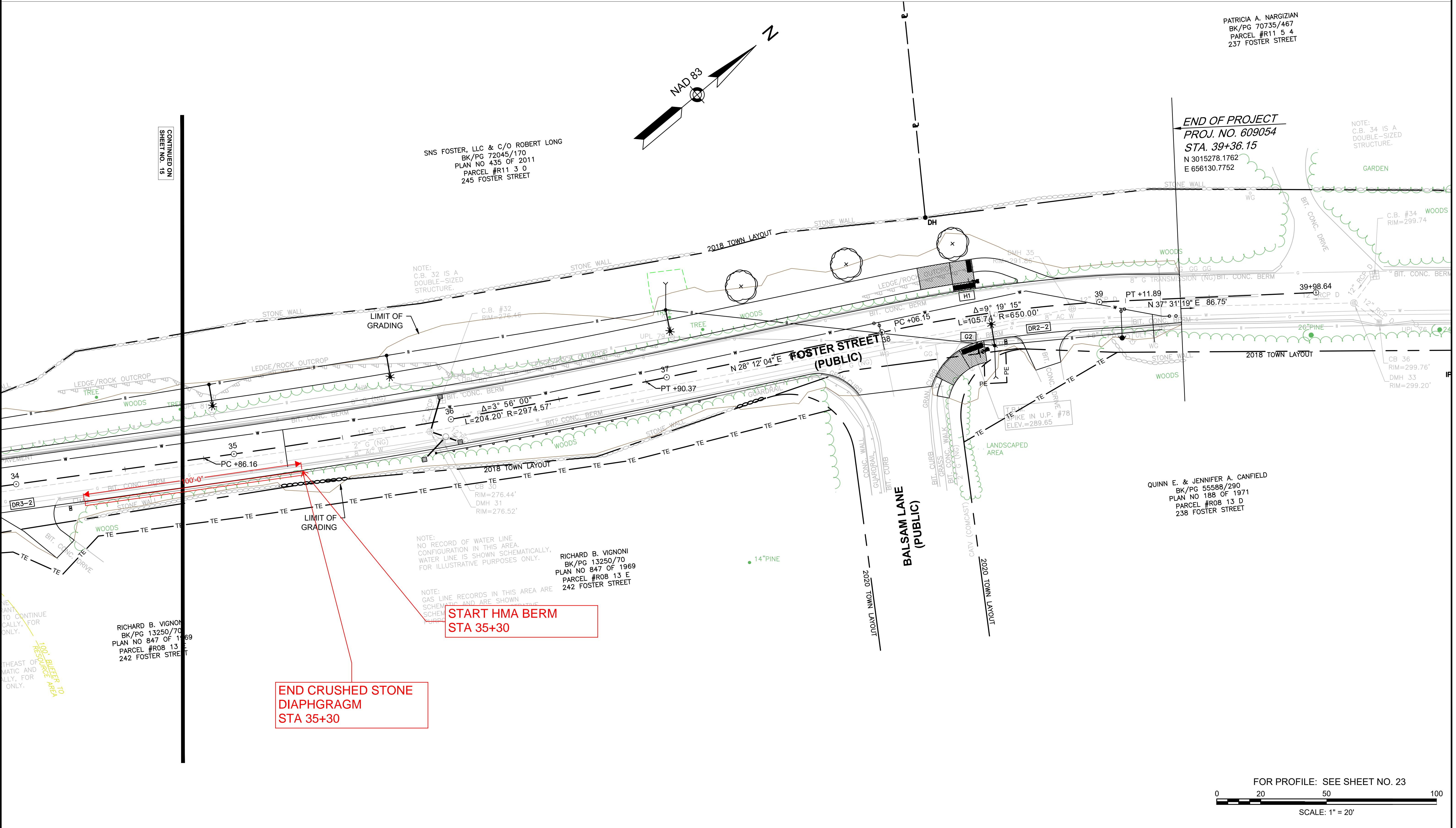
██████████	PERM IMPACT TO BUFFER ZONE
██████████	TEMP IMPACT TO BUFFER ZONE
██████████	PERM IMPACT TO 50-FT NO DISTURB
██████████	TEMP IMPACT TO 50-FT NO DISTURB
██████████	IMPERVIOUS REMOVED FROM 50-FT NO DISTURB
██████████	IMPERVIOUS REMOVED FROM BUFFER ZONE

LITTLETON  
RECONSTRUCTION OF FOSTER STREET

STATE	FED. AID PROJ. NO.	HEET NO.	TOTAL SHEETS
MA	STP/CMQ/TAP-003(037)X	E8	128

PROJECT FILE NO. 609054

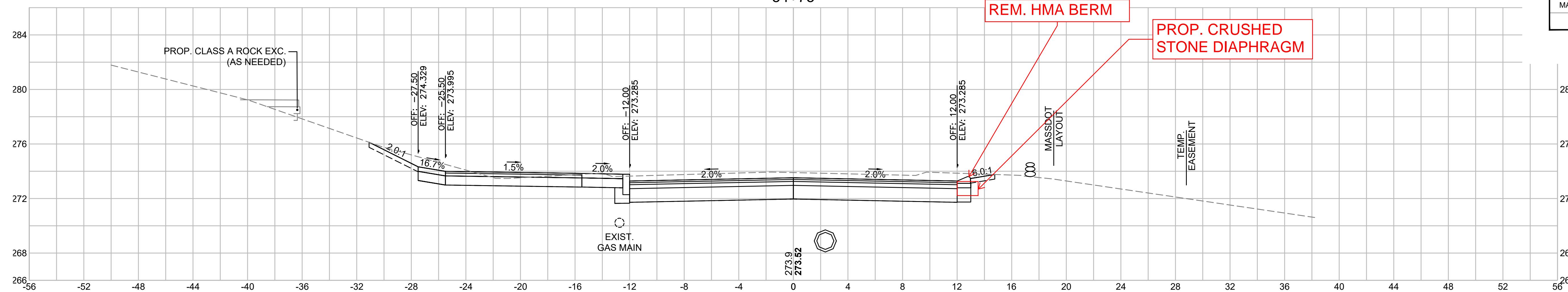
ENVIRONMENTAL PLANS



# LITTLETON RECONSTRUCTION OF FOSTER STREET

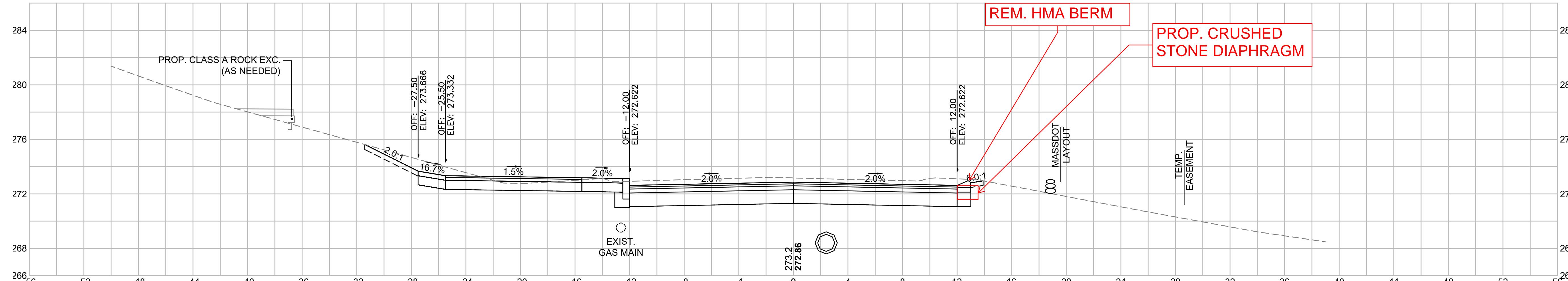
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP/CMQ/TAP-0033(037)X	121	128
PROJECT FILE NO.		609054	

## CROSS SECTIONS



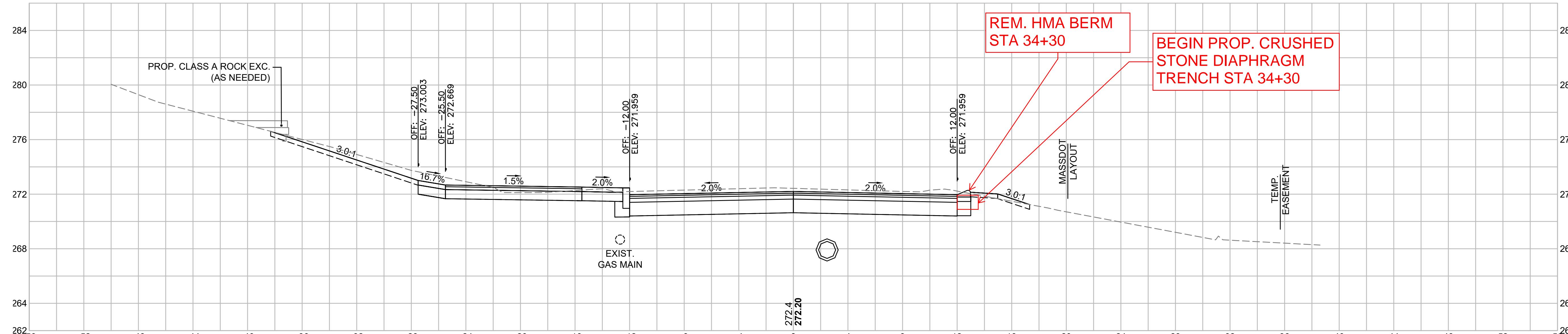
<b>Total Volume at Station 34+75.00</b>	
Cut Area	68.933
Fill Area	0.000
Cut Vol	63.0
Fill Vol	0.0
Cum Cut Vol	6935.3
Cum Fill Vol	337.3
Net Vol	6598.0

34+50



<b>Total Volume at Station 34+50.00</b>	
Cut Area	67.132
Fill Area	0.000
Cut Vol	62.1
Fill Vol	0.0
Cum Cut Vol	6872.3
Cum Fill Vol	337.3
Net Vol	6535.0

34+25



<b>Total Volume at Station 34+25.00</b>	
Cut Area	66.905
Fill Area	0.001
Cut Vol	40.6
Fill Vol	0.0
Cum Cut Vol	6810.3
Cum Fill Vol	337.3
Net Vol	6472.9

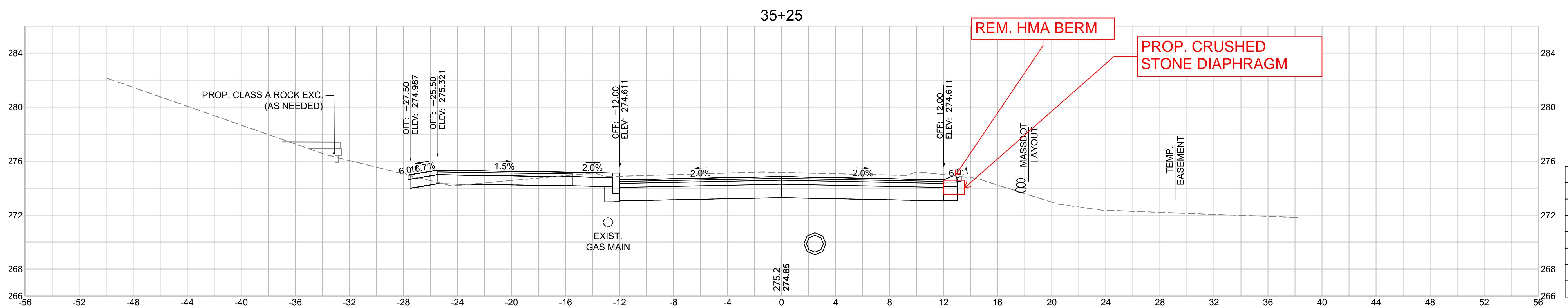
# LITTLETON RECONSTRUCTION OF FOSTER STREET

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP/CMQ/TAP-0033(037)X	122	128
PROJECT FILE NO.		609054	

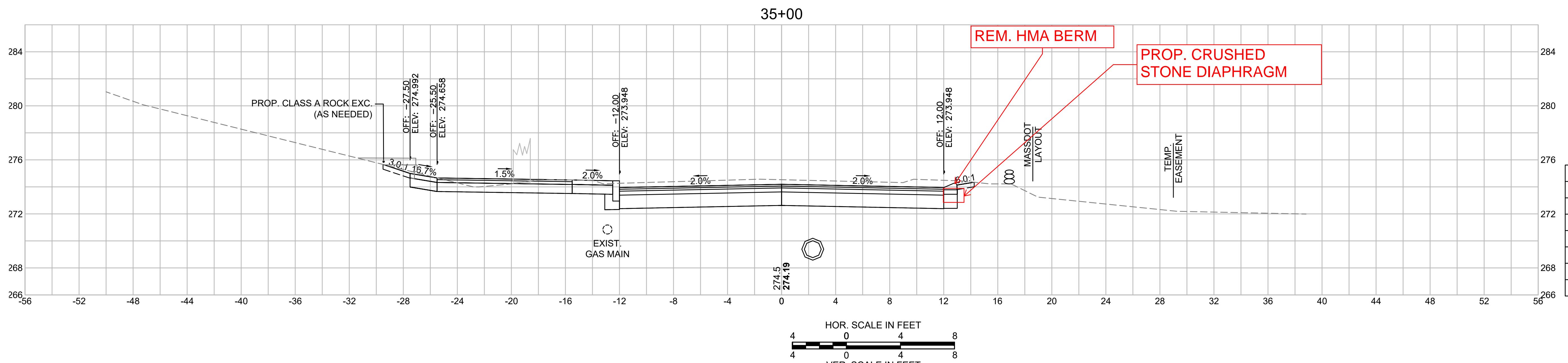
## CROSS SECTIONS

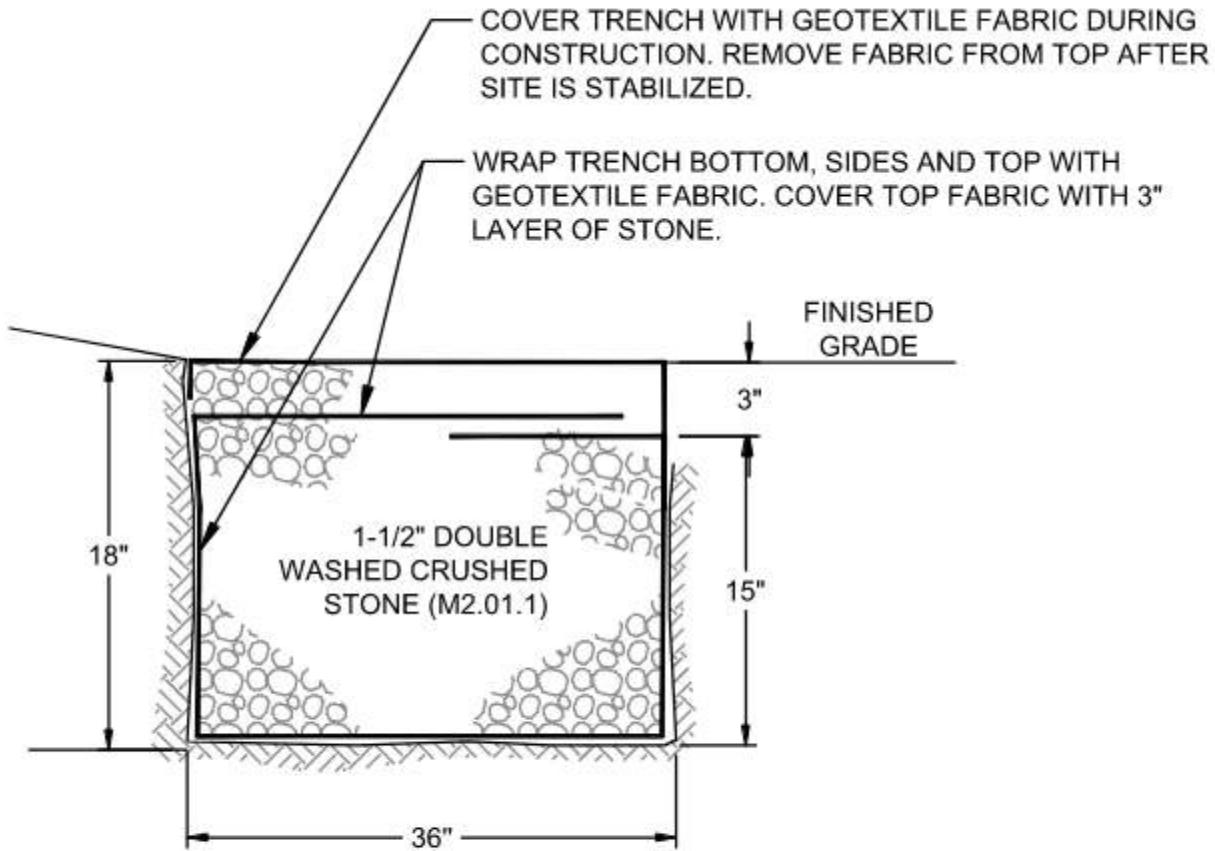


Total Volume at Station 35+50.00	
Cut Area	52.062
Fill Area	0.952
Cut Vol	49.8
Fill Vol	0.5
Cum Cut Vol	7101.8
Cum Fill Vol	337.9
Net Vol	6763.9



<b>Total Volume at Station 35+25.00</b>	
Cut Area	55.495
Fill Area	0.142
Cut Vol	55.2
Fill Vol	0.1
Cum Cut Vol	7052.0
Cum Fill Vol	337.4
Net Vol	6714.6





## CRUSHED STONE DIAPHRAGM

NOT TO SCALE

STORMWATER STORAGE VOLUME  
CALCULATIONS  
EQUIVALENCY BETWEEN 4 LEACHING CATCH  
BASINS AND A 100FT LONG CRUSHED STONE  
DIAPHRAGM TRENCH

<b>Storage Volume Calculation for Stone Diaphragm (using 18" height)</b>	
<b>H= Height (ft)</b>	1.5
<b>W =Width (ft)</b>	3
<b>L= Length (ft)</b>	100
<b>e=Void Ratio</b>	0.40
<b>Storage Volume formula</b>	$=H*W*L*e$
<b>Storage Volume of Stone Diaphragm (CF)</b>	180

Calculated by: AK

<b>Storage Volume Calculation for MassDOT Standard Leaching Catch Basin</b>	
<b>H= Height from invert out pipe to bottom of sump(ft)</b>	3
<b>R =Radius (ft)</b>	2
<b>Volume of a cylinder formula</b>	$=\pi \cdot R^2 \cdot H$
<b>Storage Volume of Each Leaching Basin (CF)</b>	37.7
<b>Number of Leaching Basins Not Installed</b>	4
<b>Total Cumulative Storage Volume of Leaching Basins Not Installed (CF)</b>	150.7